

01 FOUNDATION PLAN  
S2.00 SCALE: 1/4" = 1'-0"

#### GENERAL NOTES

##### MISCELLANEOUS

- CONTRACTOR SHALL VERIFY CONDITIONS IN THE FIELD AND IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER OF ANY CONDITIONS NOT AS ASSUMED. HE SHALL TAKE FIELD MEASUREMENTS AS REQUIRED AND BE RESPONSIBLE FOR THE SAME.
- CONTRACTOR SHALL COORDINATE WITH ALL RELATED TRADES FOR DETAILING, FABRICATION AND ERECTION PRIOR TO SUBMITTING SHOP DRAWINGS FOR APPROVAL.
- ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, ETC. REQUIREMENTS. DISCREPANCIES AND/OR INTERFERENCES SHALL BE REPORTED TO THE ARCHITECT/ENGINEER IMMEDIATELY.
- NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS OR AFTER APPROVAL FROM THE STRUCTURAL ENGINEER.
- CONTRACTOR SHALL FOLLOW THE TYPICAL DETAILS FOR TYPICAL CONDITIONS. THE TYPICAL DETAILS SHOWN ON THESE DRAWINGS APPLY THROUGHOUT THE BUILDING. UNLESS NOTED OTHERWISE NOTED.
- DESIGN REACTIONS AND SUPPORT DETAILS FOR ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT IS BASED UPON AVAILABLE MANUFACTURER INFORMATION. SUPPORT CONDITIONS MAY NEED TO BE REVISED BASED UPON ACTUAL SUPPLIED EQUIPMENT AND SUPPORT DETAILS.

##### FOUNDATIONS

- SPREAD FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OF 1,500 PSF.
- A REGISTERED PROFESSIONAL, GEOTECHNICAL ENGINEER OR COUNTY INSPECTOR SHALL VERIFY ALL PLACEMENTS OF FILL AND THAT THE FOUNDATION BEARING MATERIALS MEET OR EXCEED THE DESIGN REQUIREMENTS STATED ON THE DRAWINGS PRIOR TO PLACING ANY FOUNDATIONS.
- THE BOTTOMS OF EXTERIOR FOOTINGS SHALL BE 30 IN. MINIMUM BELOW FINISHED GRADE.
- EDGES OF FOOTINGS SHALL NOT BE PLACED AT A GREATER THAN 1 (VERTICAL) TO 2 (HORIZONTAL) SLOPE WITH RESPECT TO ANY ADJACENT FOOTING OR EXCAVATION.
- ADJACENT COLUMN FOOTINGS THAT ADJUT SHALL BE SEPERATED BY A PAPER JOINT.

##### FOUNDATION MATERIALS

- CONCRETE PLACEMENT SHALL CONFORM TO ACI 318 AND SHALL BE NORMAL WEIGHT HAVING A MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH AS FOLLOWS:

SPREAD AND WALL FOOTINGS	3500 PSI
SLAB ON GRADE	3500 PSI

- CONCRETE REINFORCING SHALL CONFORM TO THE FOLLOWING DESIGNATIONS:

DEFORMED BARS	ASTM A615,
GRADE 60	
WELDED WIRE FABRIC	ASTM A185

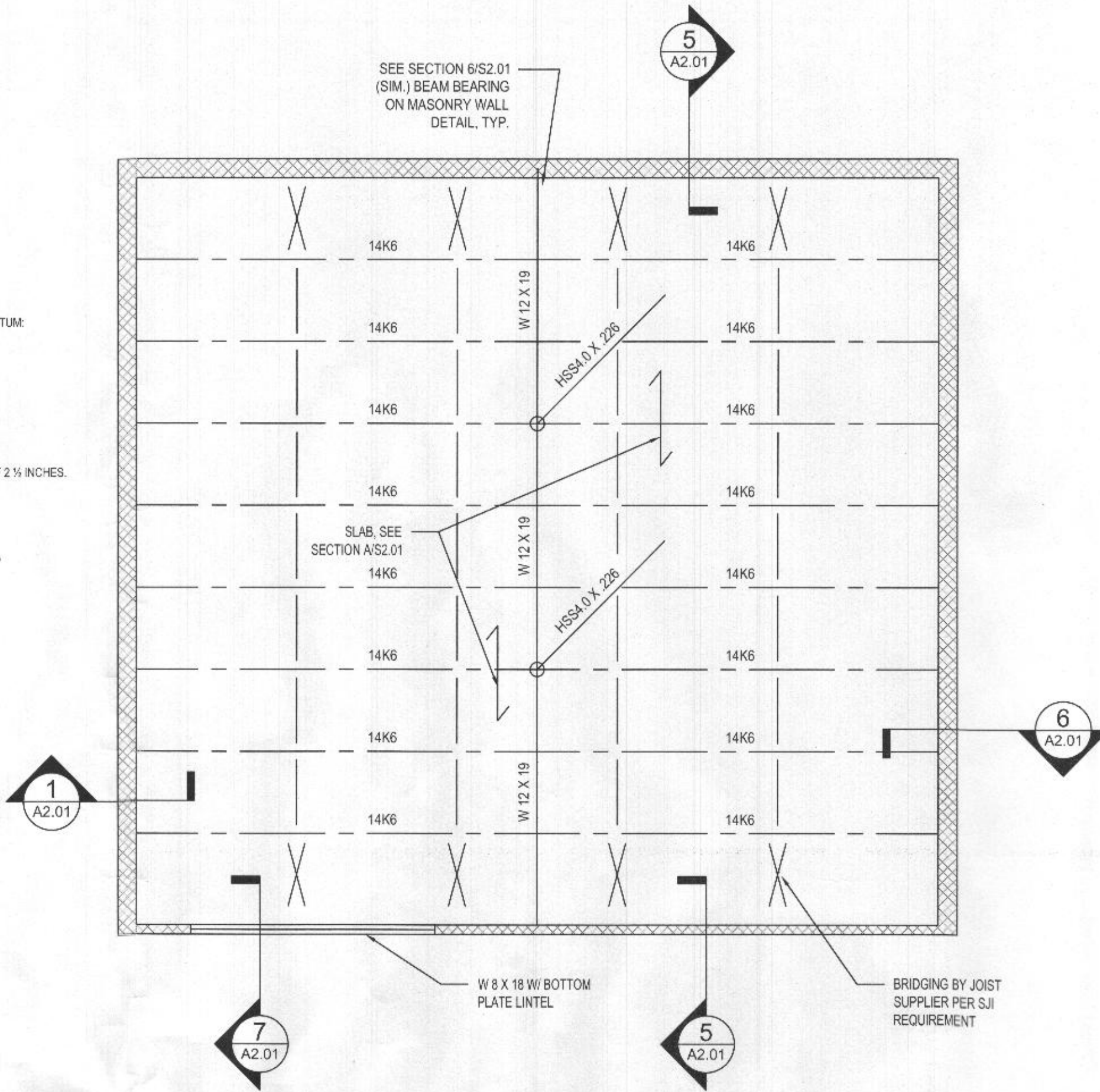
- Concrete mix design shall be based on laboratory trial batch method described in ACI-318. Concrete shall also conform to the FOLLOWING REQUIREMENTS:

Watercement Ratio	0.5 maximum
Cement Factor	470 cubic yards minimum

- CONCRETE EXPOSED TO THE ATMOSPHERE WILL BE AIR ENTRAINED 5% +/- 1%.
- SLUMP AS REQUIRED BY ACI 318.
- NO CALCIUM CHLORIDE IS PERMITTED IN THE CONCRETE.
- ALL STRUCTURAL MEMBERS WILL BE POURED TO THEIR FULL DEPTH.
- FOOTING EXCAVATIONS SHALL BE KEPT FREE OF WATER.
- SEE ARCHITECTURAL DRAWINGS FOR FINISHING AND FLATNESS REQUIREMENTS.
- ALL DEVELOPMENT LENGTHS AND SPLICES FOR DEFORMED BARS WILL BE IN ACCORDANCE WITH THE ACI BUILDING CODE REQUIREMENTS (40 BAR DIAMETERS MINIMUM). HOOKS SHALL BE STANDARD HOOKS, UNO. LAP WELDED WIRE FABRIC SUCH THAT THE OVERLAP OF THE OUTERMOST CROSS-WIRES OF EACH ADJOINING SHEET IS NOT LESS THAN THE SPACING OF THE CROSS-WIRES PLUS TWO IN. UNO.
- REINFORCING FOR SLABS ON GRADE WHERE NOT OTHERWISE SPECIFIED SHALL BE 6#6-W2 1W2.1 WWF.

##### GARAGE FLOOR PLAN NOTES

- ELEVATIONS ARE NOTED AS FOLLOWS, MEASURED FROM THE LEVEL 2 REFERENCE DATUM:  
T/S LAB (+/-X'-X") INDICATES TOP OF FRAMED CONCRETE SLAB  
(+/-X'-X") INDICATES TOP OF STEEL
- TOP OF CONCRETE SLAB SHALL BE AT T/S LAB (0'-0"), SEE ARCHITECTURAL DRAWINGS.
- TOP OF STEEL SHALL BE AT (0'-4"), UNO. - SEE SECTION A/S2.01
- STEEL BEAMS SUPPORTING K-JOISTS WILL BE LOWERED BY THE JOIST SEAT DEPTH OF 2 1/2 INCHES.
- FLOOR CONSTRUCTION SHALL BE TYPE F-1, UNO.
- NOTATIONS ON PLAN DESIGNATE THE FOLLOWING:  
R=XX UNFACTORED BEAM VERTICAL SHEAR DESIGN REACTION, (18 KIPS MIN.)
- FILLER BEAMS/JOISTS ARE TO BE EQUALLY SPACED BETWEEN COLUMNS, UNO.



02 FLOOR FRAMING PLAN  
S2.00 SCALE: 1/4" = 1'-0"

##### STEEL JOISTS

- STEEL JOISTS AND BRIDGING SHALL CONFORM TO STEEL JOIST INSTITUTE CODE OF STANDARD PRACTICE AND STANDARD SPECIFICATIONS. BRIDGING SHALL BE DESIGNED AND SPECIFIED ON THE STEEL JOIST SHOW DRAWINGS. REFER TO THE TYPICAL DETAILS FOR CONNECTION TO STRUCTURAL MEMBERS. THE BRIDGING SHALL BE PLACED AND ANCHORED PRIOR TO INSTALLING THE DECK.
- BOTTOM CHORD EXTENSIONS SHALL HAVE A MAXIMUM SLENDERNESS RATIO, KL/R, OF 200, AND, IF INDICATED ON THE DRAWINGS, A COMPRESSIVE CAPACITY (P) BOTTOM CHORD EXTENSIONS SHALL HAVE POSITIVE ATTACHMENT TO SUPPORT BY BOLTING OR BY WELDING.
- REFER TO DESIGN CRITERIA FOR NET UPLIFT LOADING REQUIREMENTS.
- THE ENDS OF STEEL JOISTS SHALL EXTEND A MINIMUM OF 2 1/2 INCHES OVER STEEL SUPPORTS AND 4 INCHES OVER ALL OTHER SUPPORTS. FASTEN ENDS BY BOLTING OR WELDING.

##### METAL DECKING

- METAL DECKING SHALL CONFORM TO THE FOLLOWING DESIGNATIONS:  
FLOOR DECK ASTM A653, STRUCTURAL, QUALITY 50 KS

- SPECIFIED COMPOSITE FLOOR DECK GAGES ARE MINIMUM. PROVIDE GAGE CONFORMING TO SDI REQUIREMENTS FOR STRESS AND DEFLECTION DURING CONSTRUCTION, BASED ON ACTUAL SPAN LENGTHS, NUMBER OF CONTINUOUS SPANS, LOADING AND PRESENCE OR ABSENCE OF SHORING. PROVIDE SHORING IF REQUIRED, OR FURNISH HIGHER GAGE DECK IF REQUIRED TO SUPPORT APPLICABLE LOADS.
- SPECIFIED ROOF DECK HAS BEEN DESIGNED TO BE CONTINUOUS OVER 3 SPANS MINIMUM. FOR ONE OR TWO SPAN CONDITIONS, FURNISH HIGHER GAGE DECK IF REQUIRED TO SUPPORT APPLICABLE LOADS.
- METAL DECK SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES PER FOOT WIDTH:  
1-1/2" TYPE C "CONFORM" FLOOR DECK, 22 GAUGE 1 = 0.177 IN<sup>4</sup> S = 0.179 IN<sup>3</sup>

- FLOOR DECK SHALL HAVE A ZINC COATING CONFORMING TO ASTM A525.
- SEE THE TYPICAL DETAILS FOR THE CONNECTION OF THE DECK TO THE STRUCTURE.
- PROVIDE STANDARD CLOSURES, CANT STRIPS, POUR STOPS, AND OTHER ACCESSORIES AS SHOWN ON THE DRAWINGS OR AS REQUIRED.

##### MASONRY

- REINFORCED MASONRY CONSTRUCTION (CMU) SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI 530, AND SPECIFICATIONS FOR MASONRY STRUCTURES, ACI 530.1.
- CMU SHALL BE GRADE N, 2-CELL BLOCK AND CONFORM TO ASTM C90 (NORMAL WEIGHT). PROVIDE TYPE I FOR EXTERIOR WALLS AND FOUNDATIONS. TYPE I OR TYPE II BLOCK MAY BE USED FOR INTERIOR PARTITION WALLS.
- MINIMUM COMPRESSIVE STRENGTH OF MASONRY, F<sub>M</sub> SHALL BE 1500 PSI.
- MASONRY REINFORCING SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60.
- GROUT SHALL CONFORM TO ASTM C476, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
- MORTAR SHALL CONFORM TO ASTM C270, TYPE S.
- REINFORCED VOIDS IN MASONRY AS CALLED OUT ON THE CONTRACT DOCUMENTS SHALL BE FILLED SOLID WITH GROUT (3,000 PSI PEA GRAVEL), AND A MAXIMUM OF 6 COURSE LIFTS WITH BE GROUTED AT ONE TIME.
- LAP DEFORMED BARS 48 DIAMETERS AND PLACE IN THE CENTER OF THE BLOCK, UNO.
- ALL CMU SHALL HAVE LADDER TYPE HORIZONTAL JOINT REINFORCEMENT AT 16 INCHES ON CENTER VERTICAL SPACING WITH PREFABRICATED CORNER REINFORCING. LAP HORIZONTAL REINFORCING A MINIMUM OF 6 INCHES. PROVIDE AN ADDITIONAL ROW OF REINFORCING ABOVE AND BELOW ALL OPENINGS AND EXTEND 2 FEET BEYOND JAMBS. STOP HORIZONTAL REINFORCING AT EACH SIDE OF A VERTICAL CONNECTION JOINT.

##### LOOSE LINTELS FOR OPENINGS IN MASONRY WALLS

- PROVIDE LOOSE LINTELS OVER PENETRATIONS IN NEW MASONRY WALLS AND NEW PENETRATIONS IN EXISTING MASONRY WALLS AT DOORS, WINDOWS, MECHANICAL AND ELECTRICAL SERVICES AND EQUIPMENT, ETC., UNO.
- PROVIDE ONE STEEL ANGLE FOR EACH 4 IN. OF MASONRY THICKNESS BEARING 6 IN. MINIMUM ON A FULL MORTAR BED AS FOLLOWS:

OPENINGS UP TO 3 FT.	L3-12x3-12x5/16
OPENINGS >3 FT. TO 5 FT.	L4x3-12x5/16, LONG LEG VERTICAL (LLV)
OPENINGS >5 FT. TO 8 FT.	L6x3-12x5/16, (LLV)

- WHERE REQUIRED FOR ARCHITECTURAL REASONS, OR AS NOTED, PROVIDE PRECAST CONCRETE LINTELS BEARING 8 IN. MINIMUM ON A FULL MORTAR BED AS FOLLOWS:

4" WALLS (8" MAX OPENING)	4"x8", REINFORCED WITH 1#3 TOP & 1#5
6" WALLS (8" MAX OPENING)	6"x8", REINFORCED WITH 1#3 TOP & 1#5
8" WALLS (8" MAX OPENING)	8"x8", REINFORCED WITH 2#3 TOP & 2#5

- WHEN WALLS ARE PRESENT THAT ARE THICKER THAN 8" USE COMBINATION OF 4", 6", AND 8" PRECAST CONCRETE LINTELS

##### WOOD

PLEASE SUBMITT SUBSTITUTIONS (PRODUCT INFORMATION) TO THE STRUCTURAL ENGINEER FOR APPROVAL.

- STRUCTURAL SAWN LUMBER SHALL BE OF NOMINAL SIZE CROSS SECTIONS AS SHOWN ON THE PLANS AND SECTIONS AND SHALL BE EITHER SPF NO. 1-2 OR HEM FIR NO.2 WITH THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES:

SPF NO. 1-2		
F <sub>b</sub>	875 PSI	
F <sub>t</sub>	450 PSI	
F <sub>v</sub>	70 PSI	
F <sub>c</sub> (PERPENDICULAR TO GRAIN)	405 PSI	
F <sub>c</sub> (PARALLEL TO GRAIN)	1,150 PSI	
E (MOE)	1,300,000 PSI	

- STRUCTURAL LVL's SHALL HAVE THE MINIMAL SIZE CROSS SECTIONS AS SHOWN ON THE PLANS AND SHALL HAVE THE FOLLOWING MINIMUM STRUCTURAL PROPERTIES:

LVL's		
F <sub>b</sub>	2,800 PSI	
F <sub>v</sub>	290 PSI	
E (MOE)	1,900,000 PSI	

- T/J AS CALLED OUT ON PLAN WILL BE MANUFACTURED BY TRUS JOIST MACMILLAN. SUBSTITUTIONS WILL BE OF EQUAL STRENGTH AND QUALITY AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.
- ALL MULTIPLE BEAMS, LINTELS, AND JOISTS WILL BE GLUED TOGETHER USING LIQUID NAILS AND SCREWED WITH 3 1/4 INCH LONG DECK WASTE SCREWS AT 16 INCHES O.C. STAGGERED 2 INCHES FROM THE TOP AND BOTTOM OF THE MEMBERS.
- INSTALLED 2X (SAND DEPTH AS JOIST) BRIDGING AT 8 FEET O.C. MAX. PERPENDICULAR TO THE JOIST SPAN AS SHOWN ON PLAN.
- PROVIDE SIMPSON CONNECTORS FOR ALL WOOD TO WOOD CONNECTIONS AS CALLED OUT ON PLANS. FOLLOW SIMPSON'S RECOMMENDATIONS FOR FASTENING THE CONNECTORS.

#### 6 HJFD DESIGN CRITERIA

- DEAD, LIVE, WIND AND SEISMIC DESIGN LOADS ARE IN ACCORDANCE WITH THE 2015 INTERNATIONAL RESIDENTIAL BUILDING CODE (IRC) AND THE HOWARD COUNTY BUILDING CODE. THE ROOF AND ATTIC HAS BEEN DESIGNED FOR A 30 PSF LIVE LOAD, AND THE GARAGE FLOOR HAS BEEN DESIGNED FOR A 80 PSF LIVE LOAD.

- SNOW LOADING IS BASED ON THE FOLLOWING:

GROUND SNOW LOAD	30 PSF
BUILDING CATEGORY	II
EXPOSURE FACTOR	1.0
THERMAL FACTOR	1.0
IMPORTANCE FACTOR	1.0

- WIND LOADING FOR THE MAIN WIND-FORCE RESISTING SYSTEM IS BASED ON THE FOLLOWING:

BASIC WIND SPEED (ULT.)	115 MPH
BUILDING CATEGORY	II
EXPOSURE	B
IMPORTANCE FACTOR	1.0
TOPOGRAPHICAL FACTOR	1.0
DIRECTIONALITY FACTOR	0.85

- WIND LOADING FOR COMPONENTS AND CLADDING IS BASED ON THE FOLLOWING:

BASIC WIND SPEED (ULT.)	115 MPH
BUILDING CATEGORY	II
EXPOSURE	B
IMPORTANCE FACTOR	1.0
TOPOGRAPHICAL FACTOR	1.0
DIRECTIONALITY FACTOR	1.0

- SEISMIC LOADING IS BASED ON THE FOLLOWING:

SITE CLASSIFICATION	C
0.2 SECOND SPECTRAL RESPONSE ACCELERATION, S <sub>S</sub>	0.125
1.0 SECOND SPECTRAL RESPONSE ACCELERATION, S <sub>1</sub>	0.055
SEISMIC USE GROUP	I
IMPORTANCE FACTOR	1.0
SEISMIC DESIGN CATEGORY	B

- ALL STUD BEARING WALLS WILL HAVE A DOUBLE CONTINUOUS TOP PLATES AND A SINGLE BOTTOM PLATE. SPLICES SHALL OCCUR OVER BEARING STUDS. STUD BEARING WALLS WILL HAVE BLOCKING AT THE MIDHEIGHT OF THE WALL.
- MULTIPLE BEARING STUDS SHALL BE NAILED TOGETHER WITH 10# NAILS AT 24 INCHES ON CENTER. PROVIDE CRIPPLES AS REQUIRED BETWEEN FLOORING AND SUPPORTING MEMBERS AT POINT LOADS ABOVE.
- ANCHOR BOLTS CONNECTING PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED.

##### SHEATHING

- FLOOR SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED PLYWOOD. SHEETS WILL BE INSTALLED WITH THE LONG DIMENSION ACROSS AT LEAST 3-JOISTS. FLOOR SHEATHING WILL BE GLUED WITH CONSTRUCTION ADHESIVE AND FASTENED WITH 8# NAILS SPACED AT 3 INCHES ON CENTER AT THE EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS.
- EXTERIOR WALL AND ROOF SHEATHING SHALL BE 7/16 (1/2) INCH APA RATED PLYWOOD. SHEETS WILL BE INSTALLED WITH THE LONG DIMENSION ACROSS AT LEAST 3-STUDS. SHEATHING WILL BE FASTENED WITH 8# NAILS SPACED AT 4 INCHES ON CENTER AT THE EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS.

##### PRE-ENGINEERED WOOD ROOF TRUSSES

- PRE-ENGINEERED ROOF TRUSSES SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MARYLAND. TRUSSES SHALL BE DESIGNED TO THE REQUIREMENTS OF THE TRUSS PLATE INSTITUTE. ALL APPLICABLE CODES, AND THE REQUIREMENTS NOTED ON THESE DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, WORKING POINTS AND PITCHES OF ENGINEERED ROOF TRUSSES. VERIFY ALL TRUSS DIMENSIONS PRIOR TO FABRICATION.
- ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM UNIFORM LOADS:

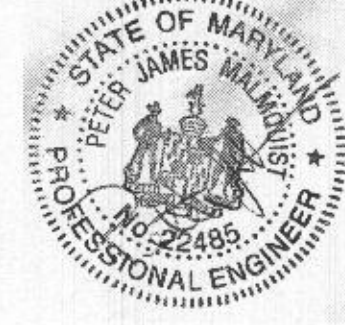
TRUSS CHORD	DEAD LOAD	LIVE LOAD	SNOW LOAD	WIND LOAD
TOP	12 PSF	30 PSF		SEE DESIGN
SEE DESIGN				
BOTTOM	8 PSF	10 PSF		CRITERIA
CRITERIA				

- ALL TRUSSES SHALL BE DESIGNED FOR SNOW LOADS INCLUDING SNOWDRIFT AND UNBALANCED SNOW LOADS AND WIND LOADS INCLUDING NET UPLIFT FORCES.
- PROVIDE 2X4 CONTINUOUS BOTTOM CHORD BRACING AT 8'-0" MAXIMUM FOR ALL WOOD TRUSSES. ADDITIONAL BRACING SHALL BE PROVIDED BY THE WOOD TRUSS MANUFACTURER AS PER THE STANDARD DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES, TPI, AS PREPARED BY THE TRUSS PLATE INSTITUTE. DESIGN TRUSS BOTTOM CHORDS AS IF UNSHEATHED, WITH BOTTOM CHORD LATERALLY SUPPORTED ONLY AT THE 2X4 CONTINUOUS BOTTOM CHORD BRACING.
- TRUSS TO STRUCTURE CONNECTION DESIGN IS THE RESPONSIBILITY OF THE TRUSS DESIGNER AND SHALL BE SHOWN ON THE TRUSS INSTALLATION DRAWINGS, AND SHALL BE SUBMITTED FOR APPROVAL.
- WOOD TRUSS ERECTOR SHALL INSTALL BRACING IN ACCORDANCE WITH THE HANDLING INSTALLATION AND BRACING REQUIREMENTS, HIB, BY THE TRUSS PLATE INSTITUTE.

#### STRUCTURAL SOLUTIONS INCORPORATED

PETER J. MALMQUIST, PE  
708 WALKER AVE.  
BALTIMORE, MD 21212  
(443) 797-7715

SEAL:



##### PROFESSIONAL CERTIFICATION:

I certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 22485, Expiration Date: 10/3/2018.

SAIZ GARAGE  
3654 SYCAMORE VALLEY RUN  
GLENWOOD, MD 21738

##### REVISIONS

NO.	DATE

##### SHEET TITLE

#### STRUCTURAL PLANS

DATE 09.25.17

SCALE AS NOTED

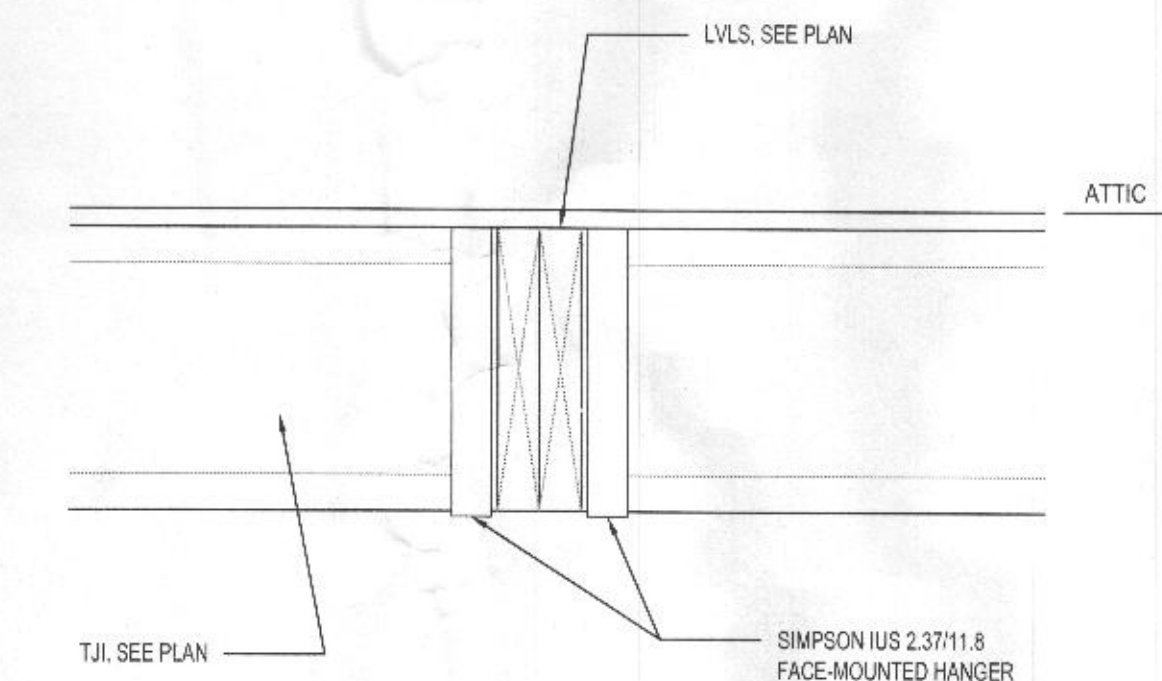
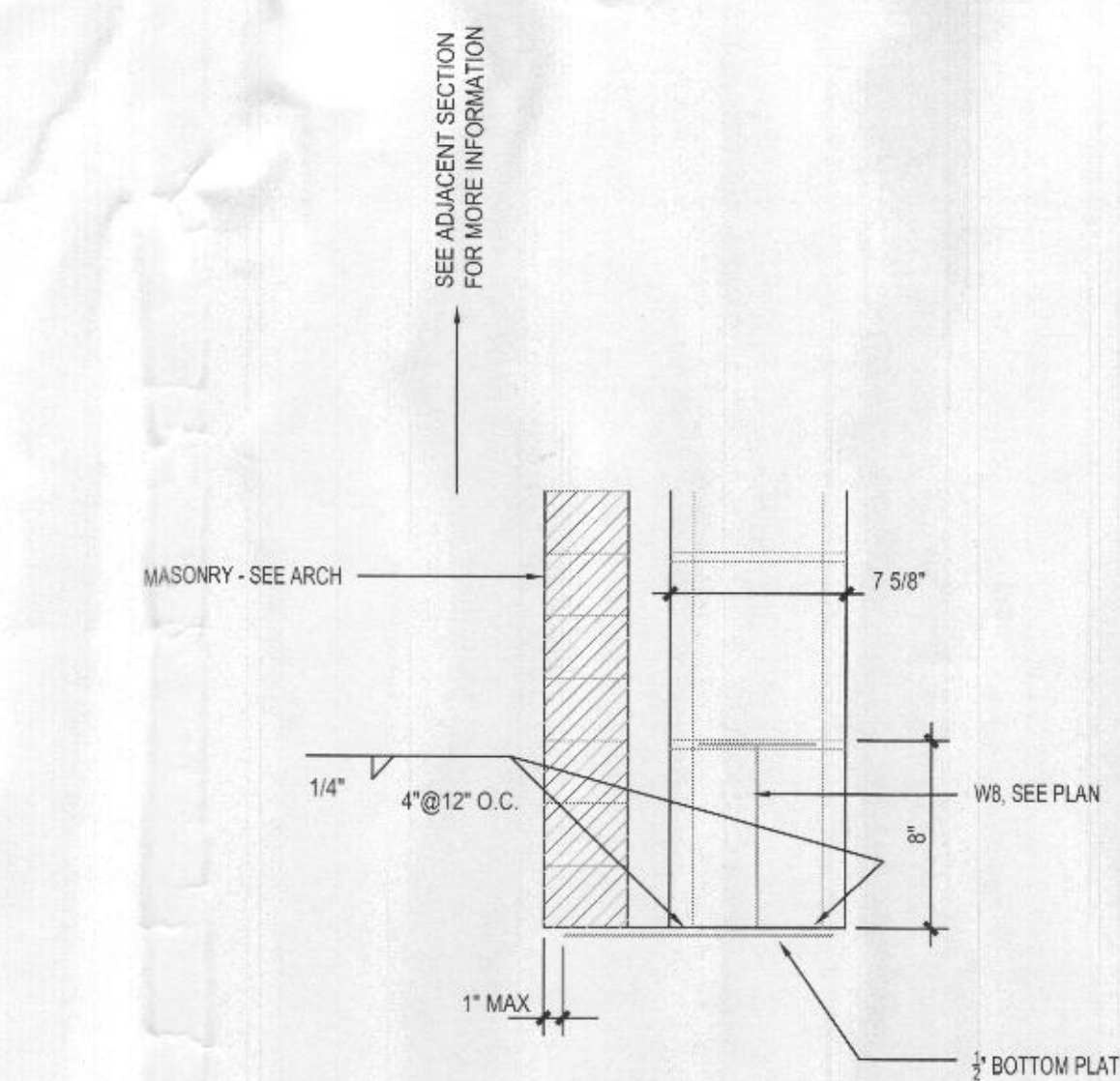
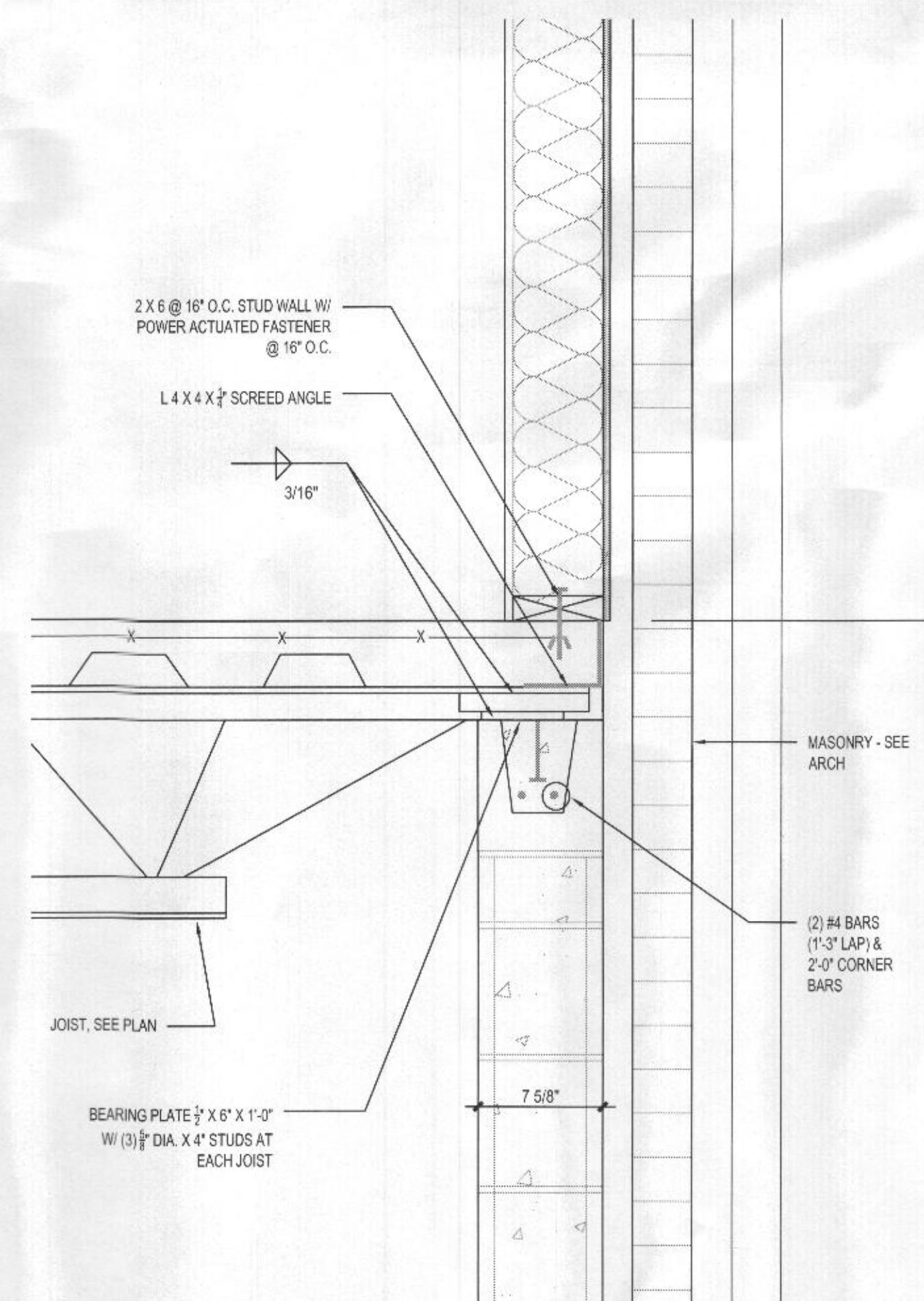
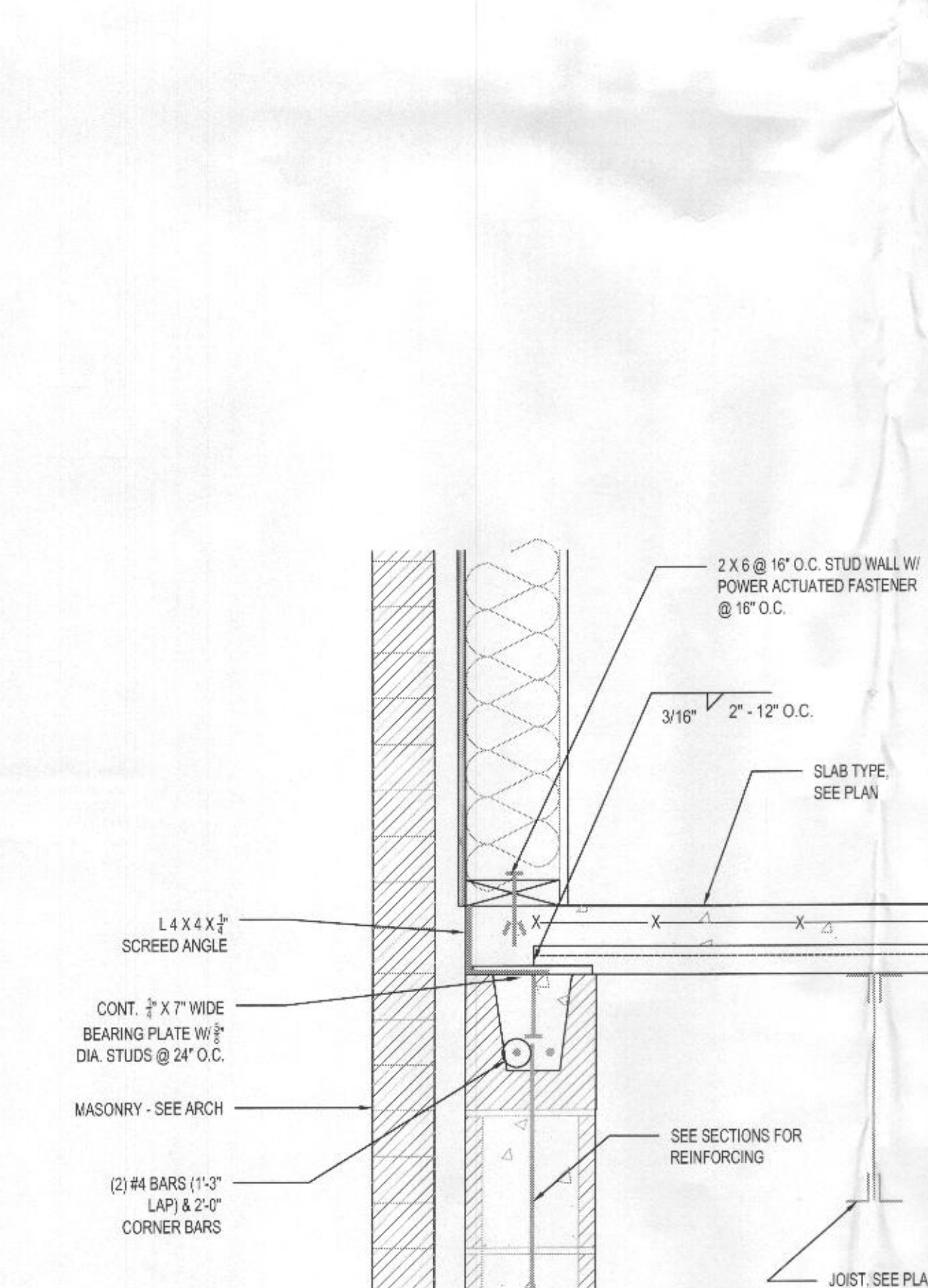
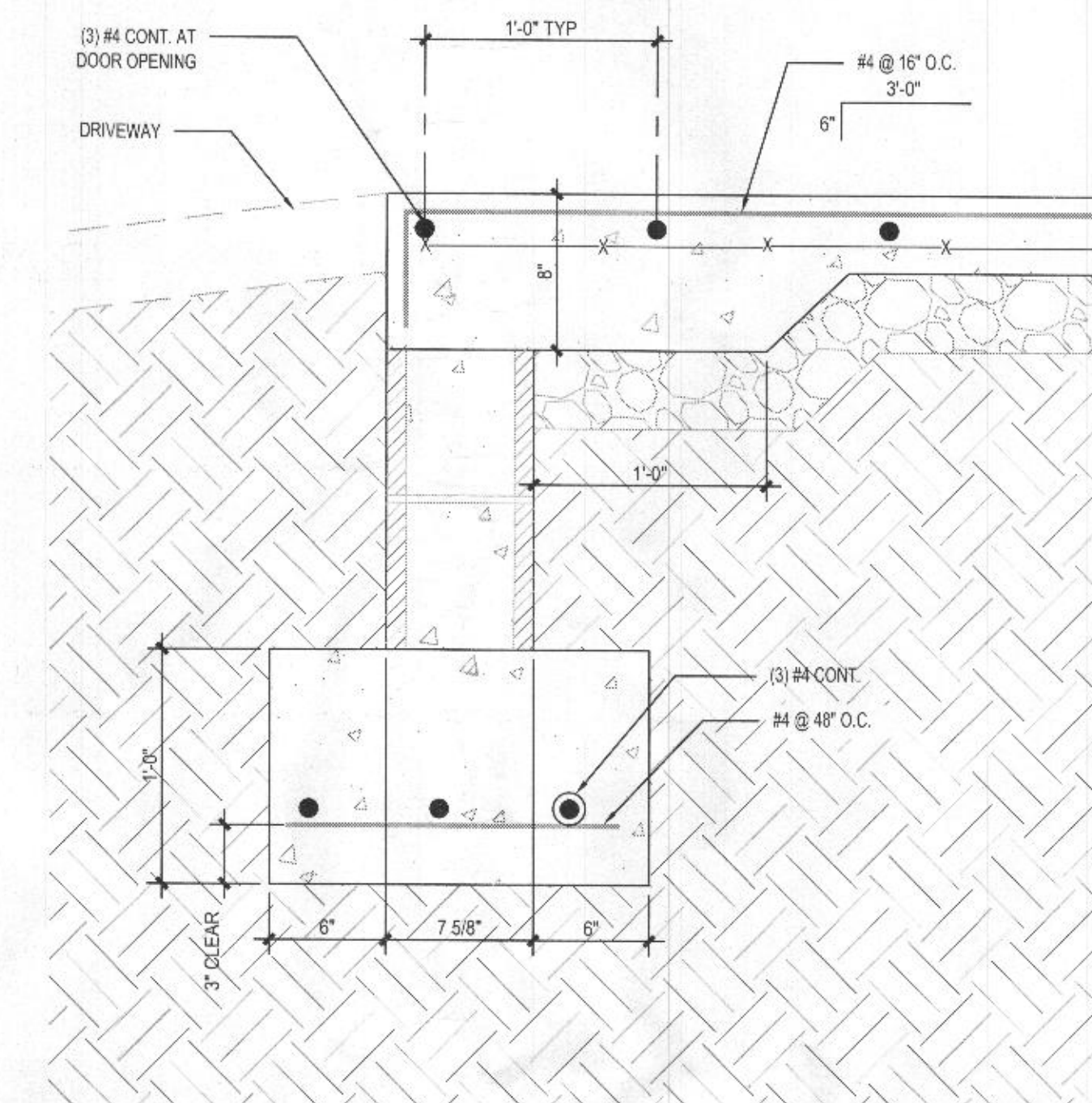
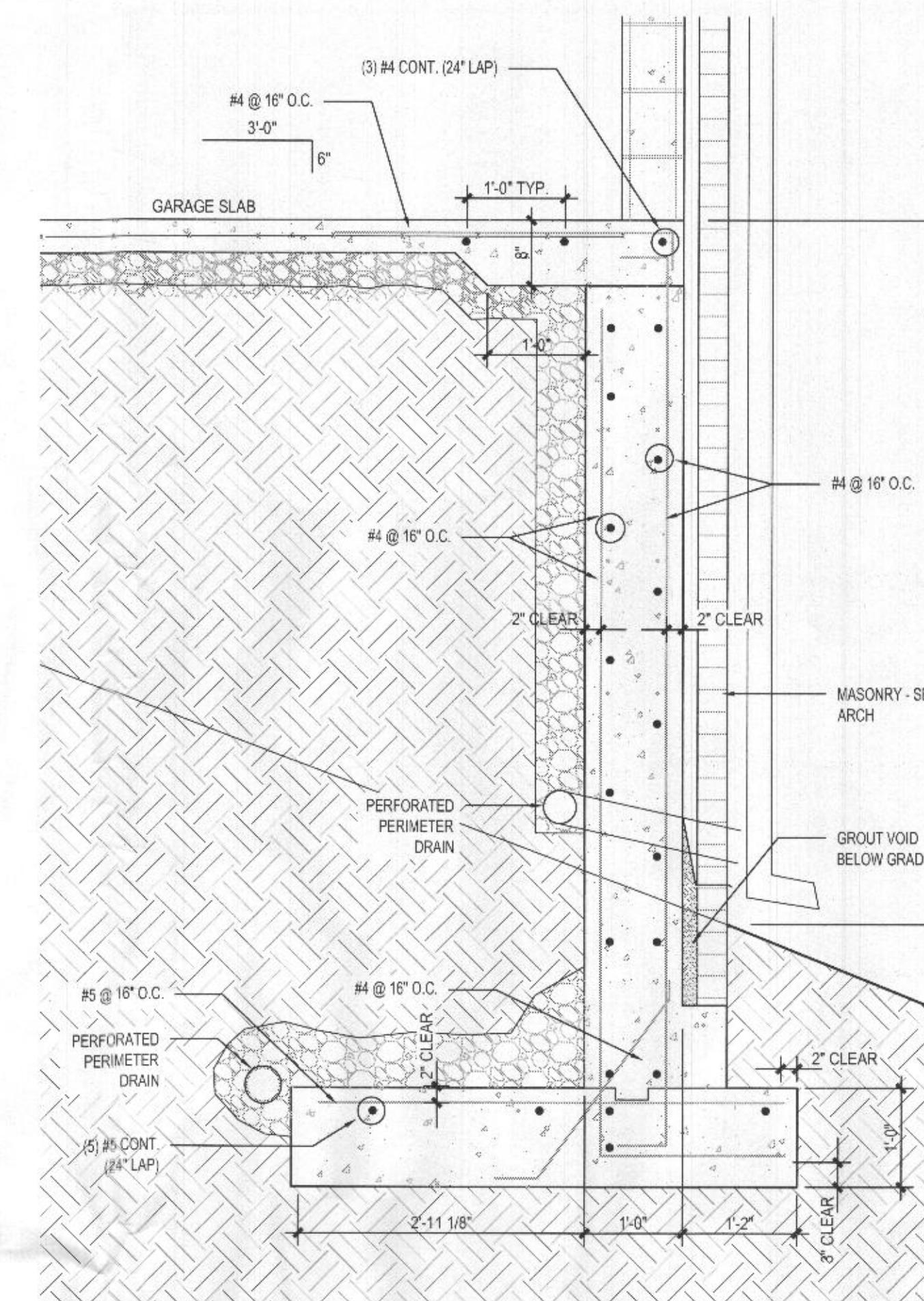
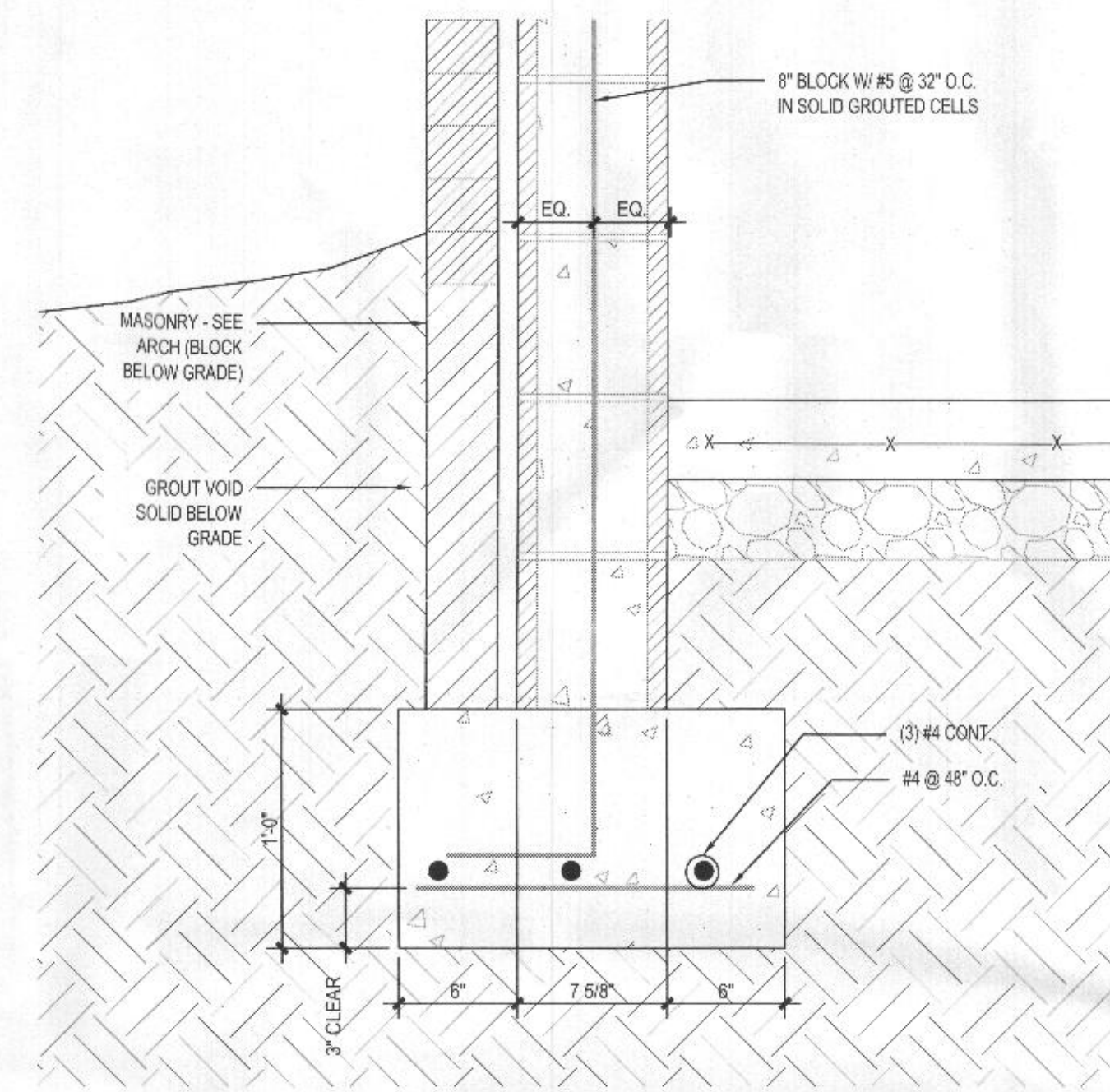
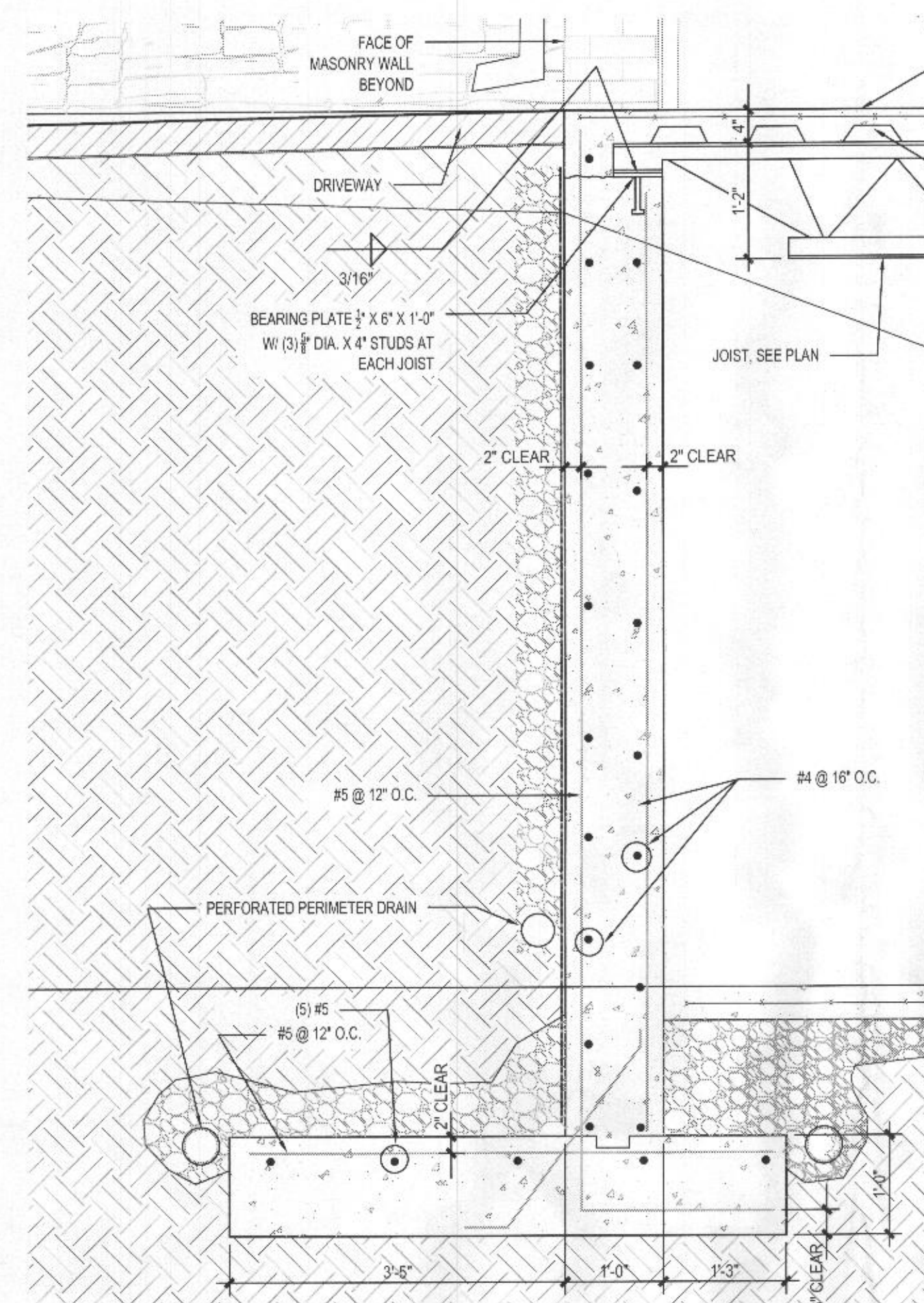
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S2.00

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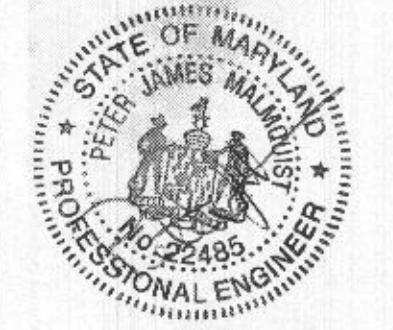




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SAIZ GARAGE  
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GLENWOOD, MD 21738

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SHEET TITLE												

## STRUCTURAL DETAILS

DATE 09.25.17

SCALE AS NOTED

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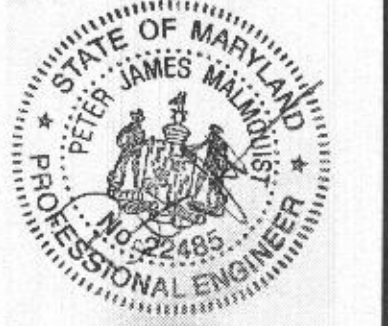
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3654 SYCAMORE VALLEY RUN  
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REVISIONS

NO. DATE

SHEET TITLE

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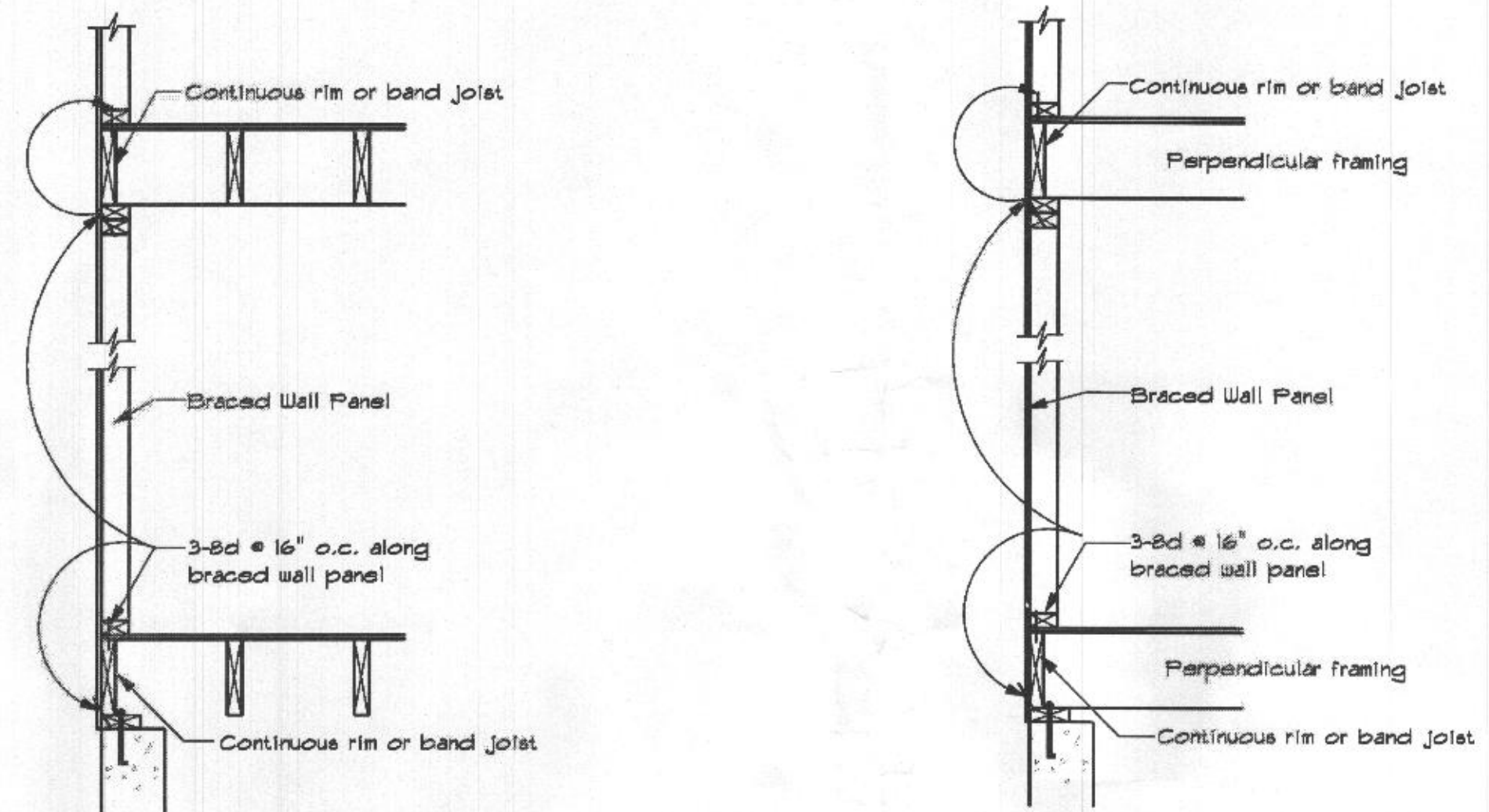
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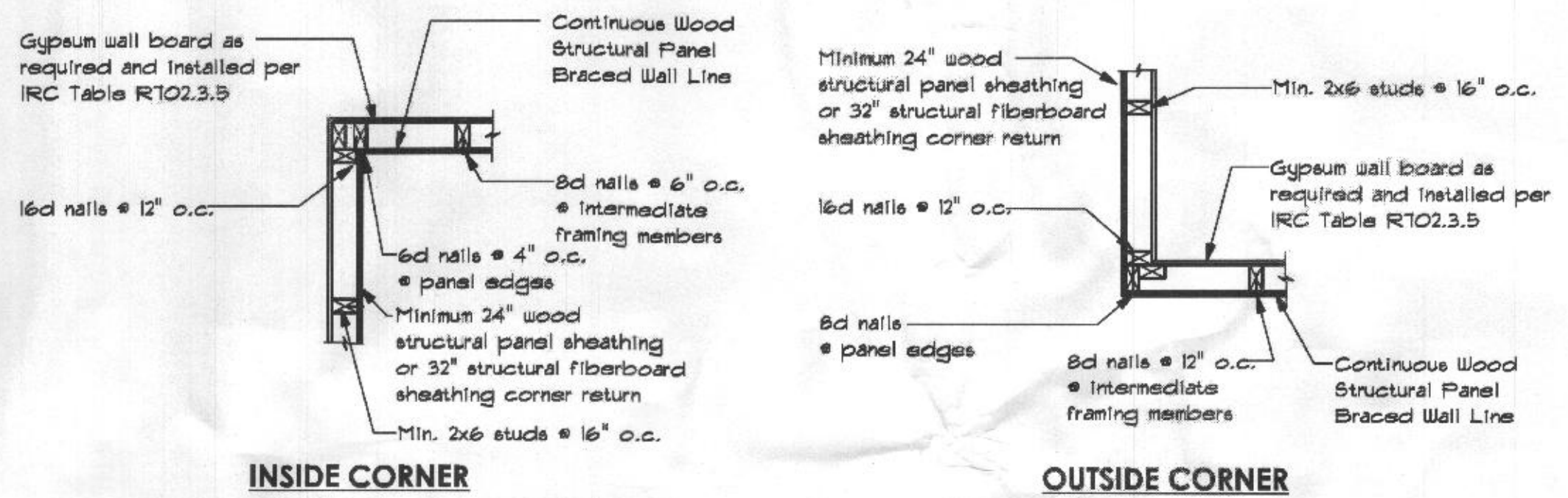


**BRACED EXTERIOR WALL PANEL CONNECTION WHEN  
PARALLEL TO FLOOR/CEILING FRAMING**

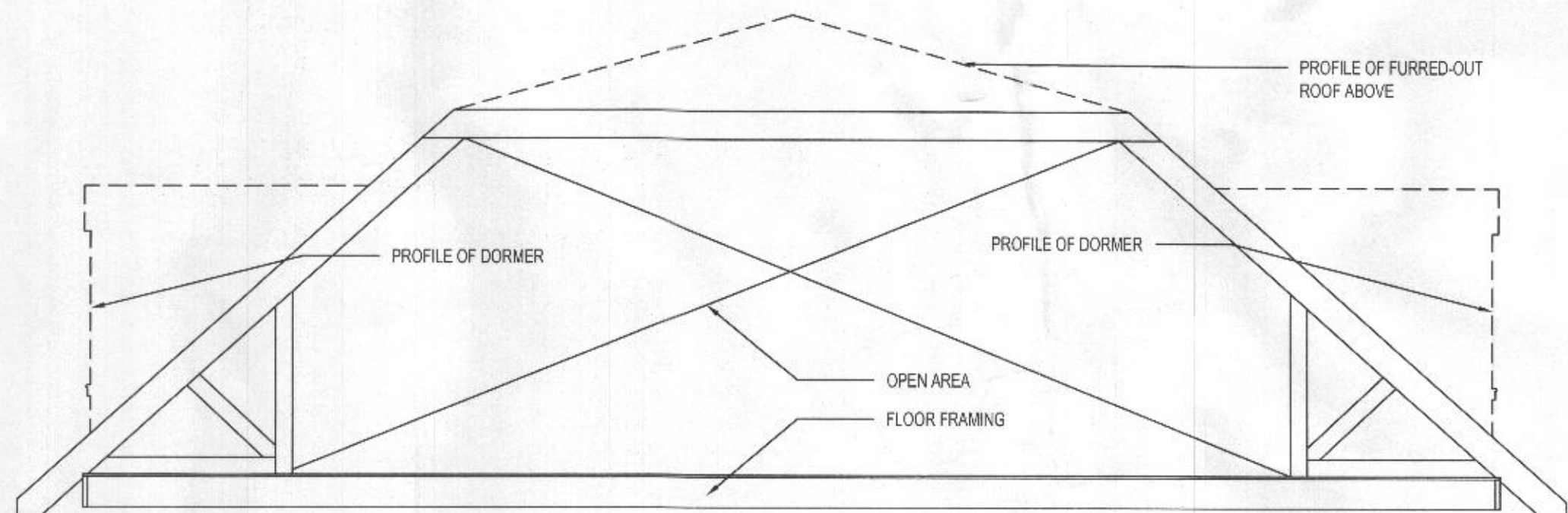
TYPICAL AT ALL EXTERIOR, PLYWOOD SHEATHED WALLS

**BRACED EXTERIOR WALL PANEL CONNECTION WHEN  
PERPENDICULAR TO FLOOR/CEILING FRAMING**

TYPICAL AT ALL EXTERIOR, PLYWOOD SHEATHED WALLS



**EXTERIOR CORNER WALL DETAILS**



**PRE-ENGINEERED WOOD ROOF TRUSSES**

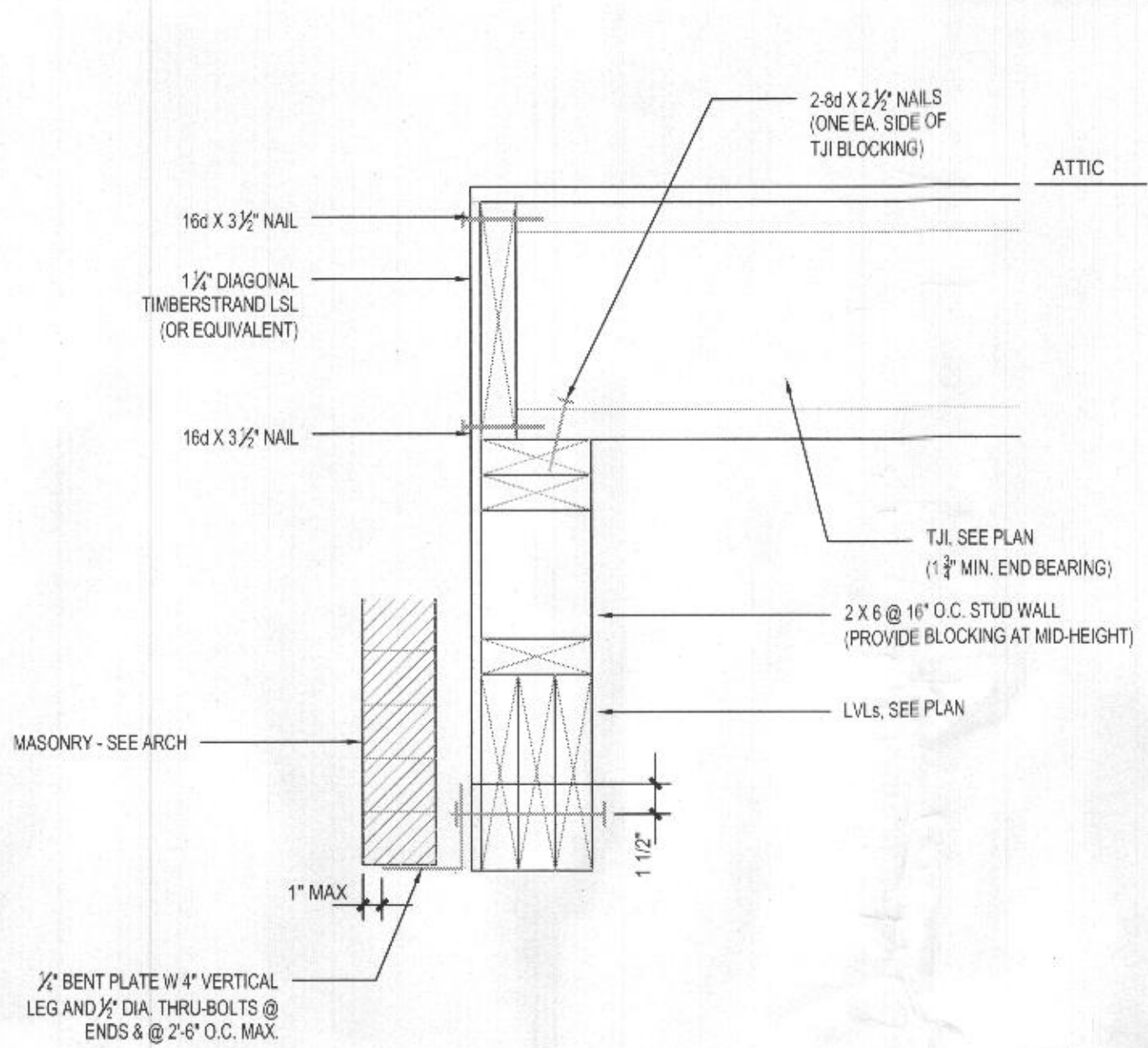
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TRUSS CHORD	DEAD LOAD	LIVE LOAD	SNOW LOAD	WIND LOAD
TOP	12 PSF	30 PSF	SEE DESIGN	SEE DESIGN
BOTTOM	5 PSF	10 PSF	CRITERIA	CRITERIA

ALL TRUSSES SHALL BE DESIGNED FOR SNOW LOADS INCLUDING SNOWDRIFT AND UNBALANCED SNOW LOADS AND WIND LOADS INCLUDING NET UPLIFT FORCES.
- PROVIDE 2X4 CONTINUOUS BOTTOM CHORD BRACING AT 8'-0" MAXIMUM FOR ALL WOOD TRUSSES. ADDITIONAL BRACING SHALL BE PROVIDED BY THE WOOD TRUSS MANUFACTURER AS PER THE STANDARD DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES, TPL AS PREPARED BY THE TRUSS PLATE INSTITUTE. DESIGN TRUSS BOTTOM CHORDS AS IF UNSHEATHED, WITH BOTTOM CHORD LATERALLY SUPPORTED ONLY AT THE 2X4 CONTINUOUS BOTTOM CHORD BRACING.
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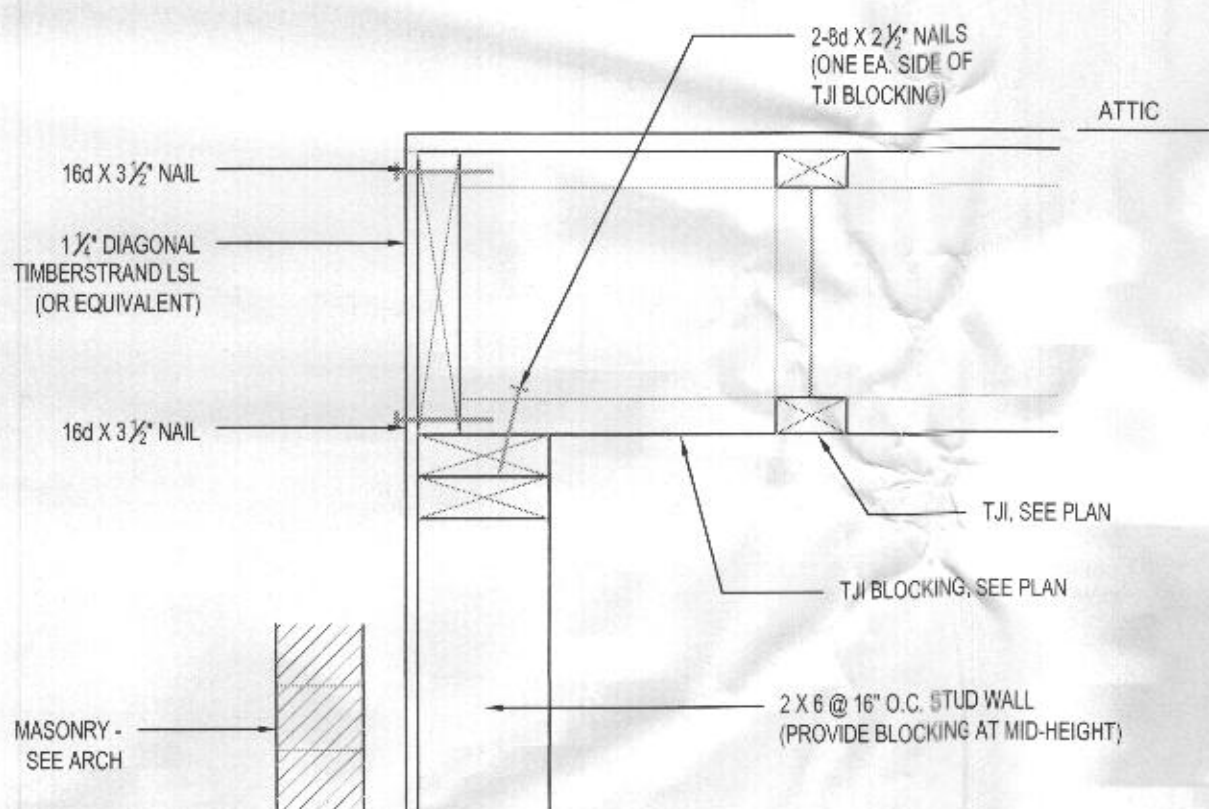
**PRE-ENGINEERED WOOD ROOF TRUSS PROFILE**

SCALE: 1/4" = 1'-0"



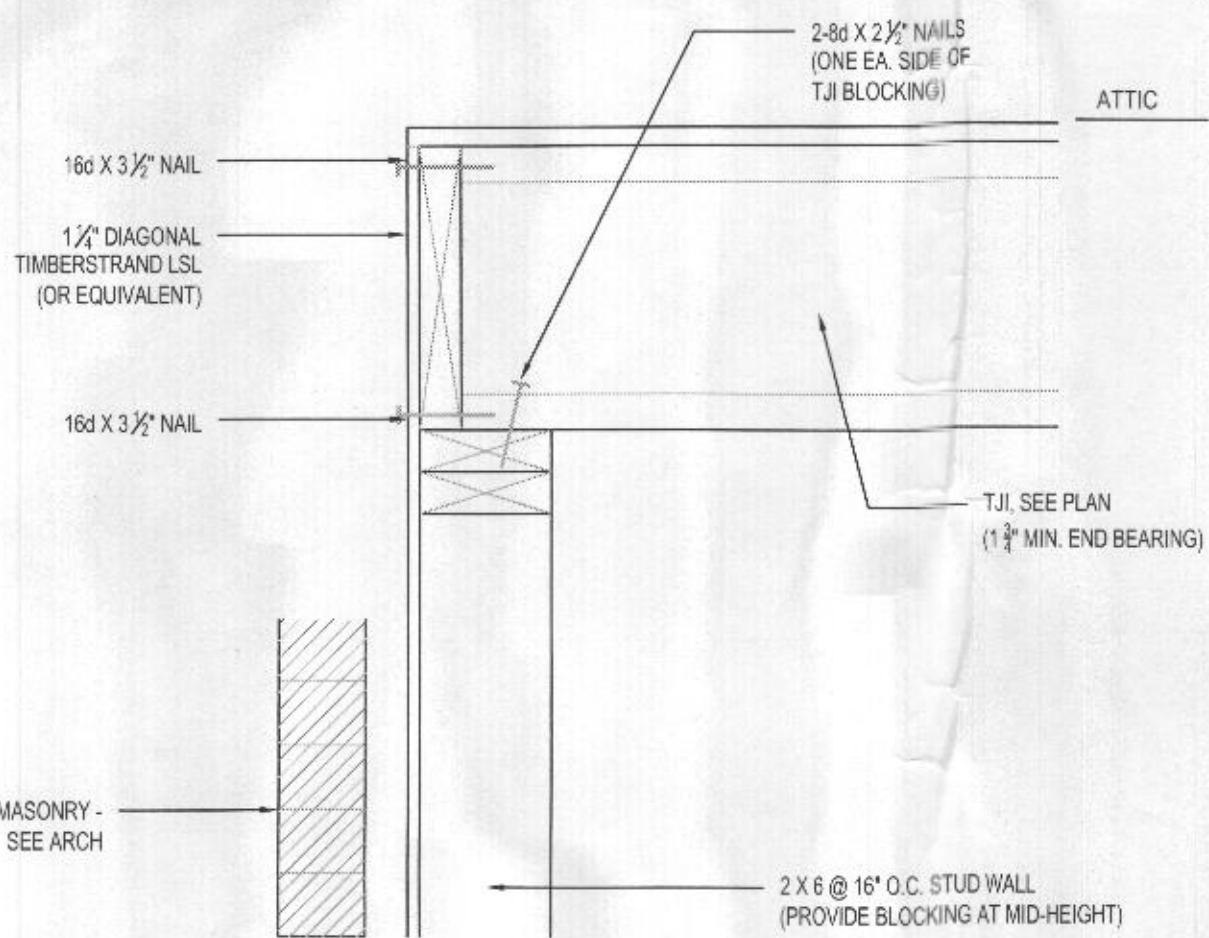
**GARAGE LINTEL DETAIL**

SCALE: 1 1/2" = 1'-0"



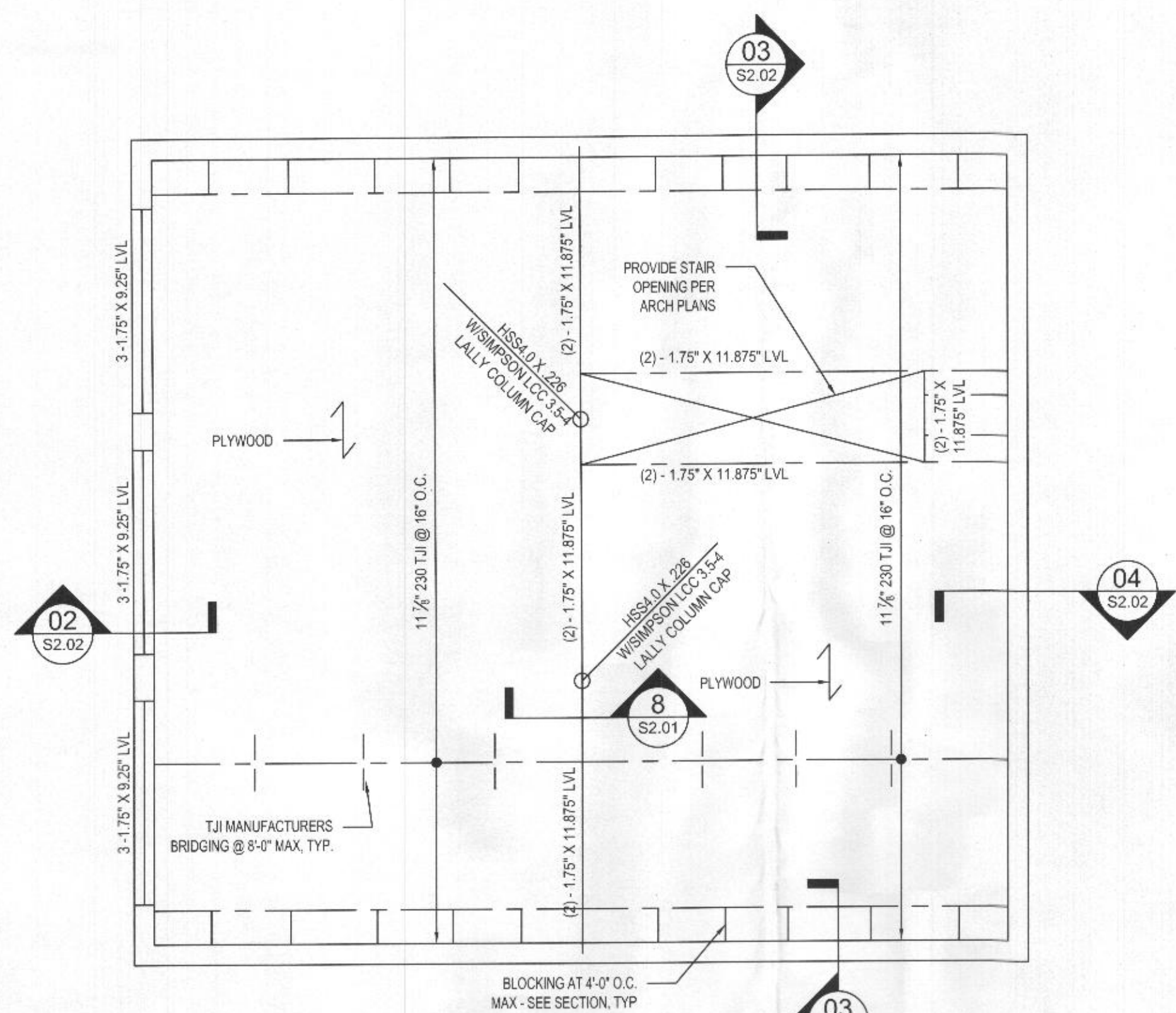
**ATTIC FRAMING DETAIL**

SCALE: 1 1/2" = 1'-0"



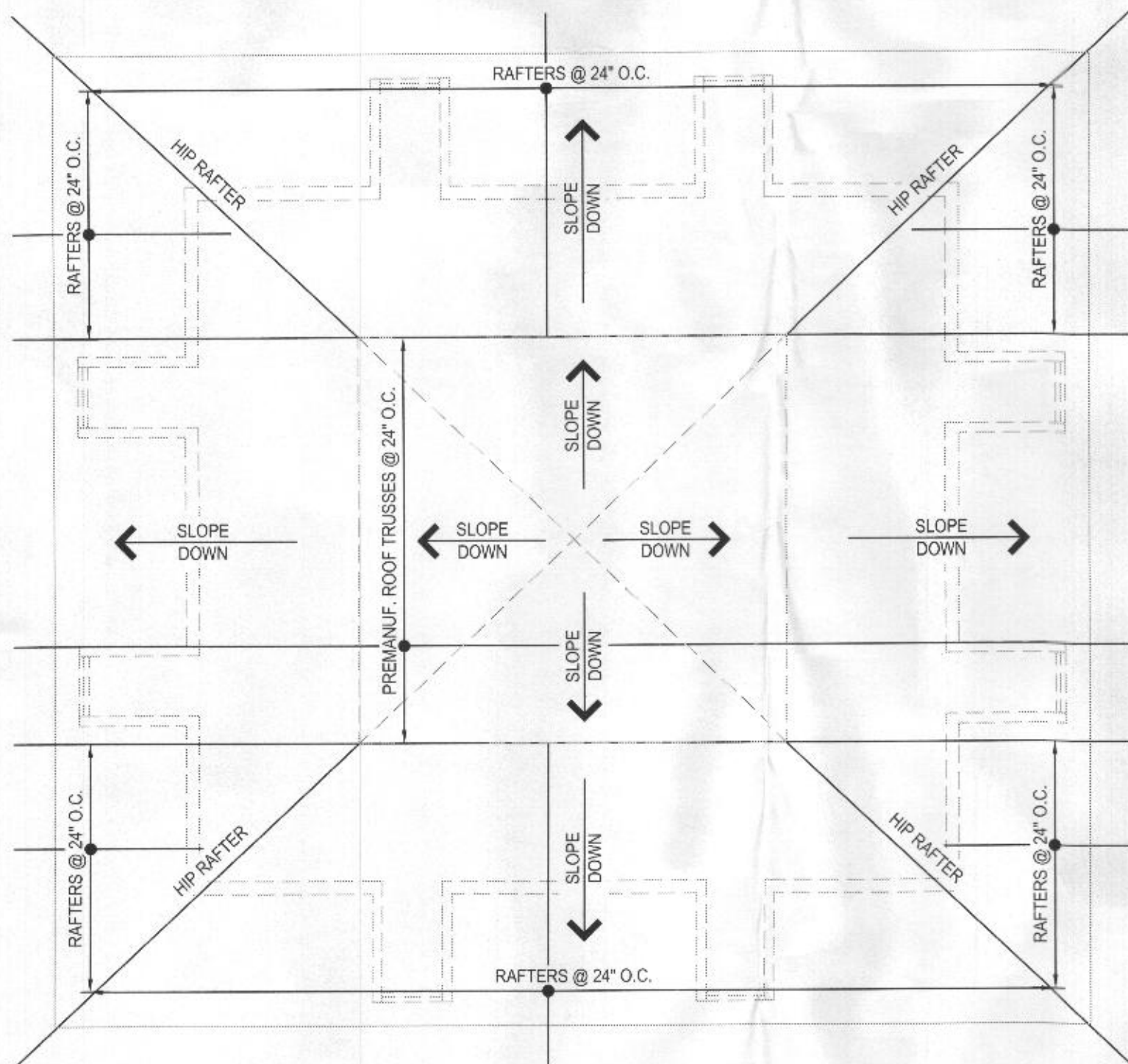
**ATTIC FRAMING DETAIL**

SCALE: 1 1/2" = 1'-0"



**ATTIC FRAMING PLAN**

SCALE: N.T.S.



**ROOF FRAMING PLAN**

SCALE: N.T.S.



M A R R E N

1016 MORTON ST  
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PROFESSIONAL CERTIFICATION:  
I certify that these documents  
were prepared or approved by me,  
and that I am a duly licensed  
architect under the laws of the  
State of Maryland, license number  
13728, expiration date 12/31/17.

NEW DETACHED GARAGE  
3654 SYCAMORE VALLEY RUN  
GLENWOOD, MD 21738

REVISIONS	
NO.	DATE
01	01.27.17

SHEET TITLE  
FLOOR PLAN  
FIREPLACE ELEVATION

DATE 10.04.17  
SCALE AS NOTED  
DRAWN BY MJM

SHEET NUMBER

A3.00



PERMIT SET