

LAYOUT 8/12/08 INSP 4 _____
INSP 2 8/13/08 INSP 5 _____
INSP 3 8/15/08 INSP 6 _____

ISSUE DATE: 8/5/2008

P 525214

APPROVAL DATE: 8/18/08

A 527937

PERMIT

TAX ID # 05-362318

ON-SITE SEWAGE DISPOSAL SYSTEM HOWARD COUNTY HEALTH DEPARTMENT BUREAU OF ENVIRONMENTAL HEALTH

Fogle's Septic Clean IS PERMITTED TO INSTALL ☒ ALTER ☐

ADDRESS: 580 Obrecht road, Sykesville PHONE NUMBER: 410-795-5670

SUBDIVISION: Oyekan Property LOT NUMBER: 4

ADDRESS: 7145 Brooks Road PROPERTY OWNER: Omololu Oyekan

SEPTIC TANK CAPACITY (GALLONS): 2000 OUTLET BAFFLE FILTER REQUIRED ☒

PUMP CHAMBER CAPACITY (GALLONS): 2250
2000 COMPARTMENTED TANK REQUIRED ☒

NUMBER OF BEDROOMS: 6

SQUARE FEET PER BEDROOM: _____

LINEAR FEET OF TRENCH REQUIRED: 380 LF

TRENCHES:	Trench to be 2.0 feet wide. Inlet 4.0 feet below original grade. Bottom maximum depth 9.0 feet below original grade. Effective area begins at _____ feet below original grade. 5.0 feet of stone below distribution pipe.
LOCATION:	Contractor will utilize either a 2 or 3 dist. box system to accomidate a total of 8 trenches that are to be installed. 6x50' trenches and 2x40' trenches are to be installed on contour. Dbox's to be place @ the center of the trenches. Leveler's are to be installed and leveled per health department. Maintain a min 1% fall on all plumbing after D. box's
NOTES:	SDA needs to be staked per approved revised Perc. Cert. plan. prior to Layout. Initial System is for conventional trenches (one system only). Repair systems 1 and 2 are Approved Sand Mounds & will need a pump. 2250g pump tank already installed. See Perc Cert Dated 8/4/08 and approved sand mound specs dated 8/5/08.

PLANS APPROVED: Kevin Wolf DATE: 8/5/2008

NOTE: PERMIT VOID AFTER 2 YEARS

NOTE: CONTRACTOR RESPONSIBLE FOR SCHEDULING A PRE-CONSTRUCTION INSPECTION FOR ALL INSTALLATIONS

NOTE: WATERTIGHT SEPTIC TANKS REQUIRED

NOTE: ALL PARTS OF SEPTIC SYSTEM SHALL BE 100 FEET FROM ANY WATER WELL

NOTE: MANHOLE RISERS REQUIRED ON ALL SEPTIC TANKS AND PUMP CHAMBERS UNLESS SPECIFICALLY AUTHORIZED

NEITHER THE HOWARD COUNTY COUNCIL NOR THE HEALTH DEPARTMENT IS
RESPONSIBLE FOR THE SUCCESSFUL OPERATION OF ANY SYSTEM
PERMITTEE RESPONSIBLE FOR OBTAINING FINAL APPROVAL ON THIS PERMIT
CALL 410-313-2640 FOR INSPECTION OF SEPTIC SYSTEM

NOT TO SCALE

See separate sheet for larger As-Built

TRENCH/RAINFIELD DATA		
WIDTH	INLET	BOTTOM
2'	4'	9'
NUMBER OF TRENCHES 8		
TOTAL LENGTH 389'		
ABSORPTION AREA 778' SW		
DISTRIBUTION BOX LEVEL All leveled		
DISTRIBUTION BOX BAFFLE Yes (x3)		
DISTRIBUTION BOX PORT Yes (x3)		

SEPTIC TANK DATA	
SEPTIC TANK 1 LEVEL	Yes
MANUFACTURER	Mayer Bros
CAPACITY	2000 GAL
SEAM LOC	Top
TANK LID DEPTH	2'
BAFFLES	Yes
BAFFLE FILTER	Yes (8")
MANHOLE LOC	Front/Rear
6" PORT LOC	none
WATERTIGHT TEST	—
SLOTTED	Yes
PUMP/SEPTIC TANK LEVEL	Yes
MANUFACTURER	Mayer Bros
CAPACITY	2,250 GAL
SEAM LOC	Top
TANK LID DEPTH	2'
BAFFLES	Front/Rear
BAFFLE FILTER	no
MANHOLE LOC	Rear
6" PORT LOC	none
WATERTIGHT TEST	—
SLOTTED	single compartment

PRE-CONSTRUCTION
8/12/08 Tanks set from previous installation.
Run gravity line from tank to Dbox w/ c/p every

60'. Utilize 3 Dboxes for an even distribution of 6x50' trenches on contour and 2x40' trenches using both directions. Keep system 25' removed from 25% slopes. (KW) 8/13/08 Top 2 Dboxes set w/ 3x50' trenches complete. OK to continue. (KW) 8/15/08 Third D box set, other 3x50' trenches completed. make sure contractor uses stone under each Dbox w/ bunched on every pipe (KW) 8/18/08 Last 2 trenches installed. Dboxes leveled out all same. OK to backfill gently around D boxes.

FINAL INSPECTOR

DATE OF APPROVAL

8/18/08



INNOVA, LTD

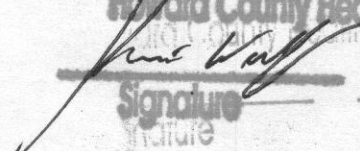
INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax
H. Dale Gray, Principal

WASTEWATER DISPOSAL SYSTEM

Approved Septic System Plan
Howard County Health Department
 8/5/08
Signature Date

Re. HCHD File A522089

Project Title: Mr. Omalalu James Oyekan

Address: 7145 Brooks Rd, Highland MD 20777

6/24/08

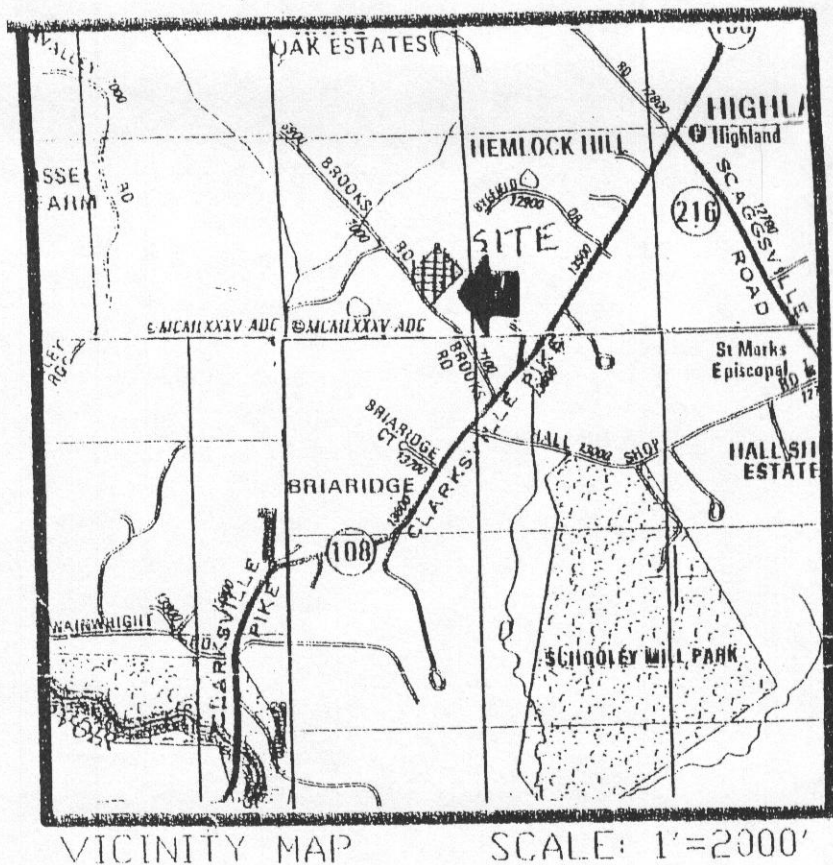
2008 JUL -7 AM 7:54

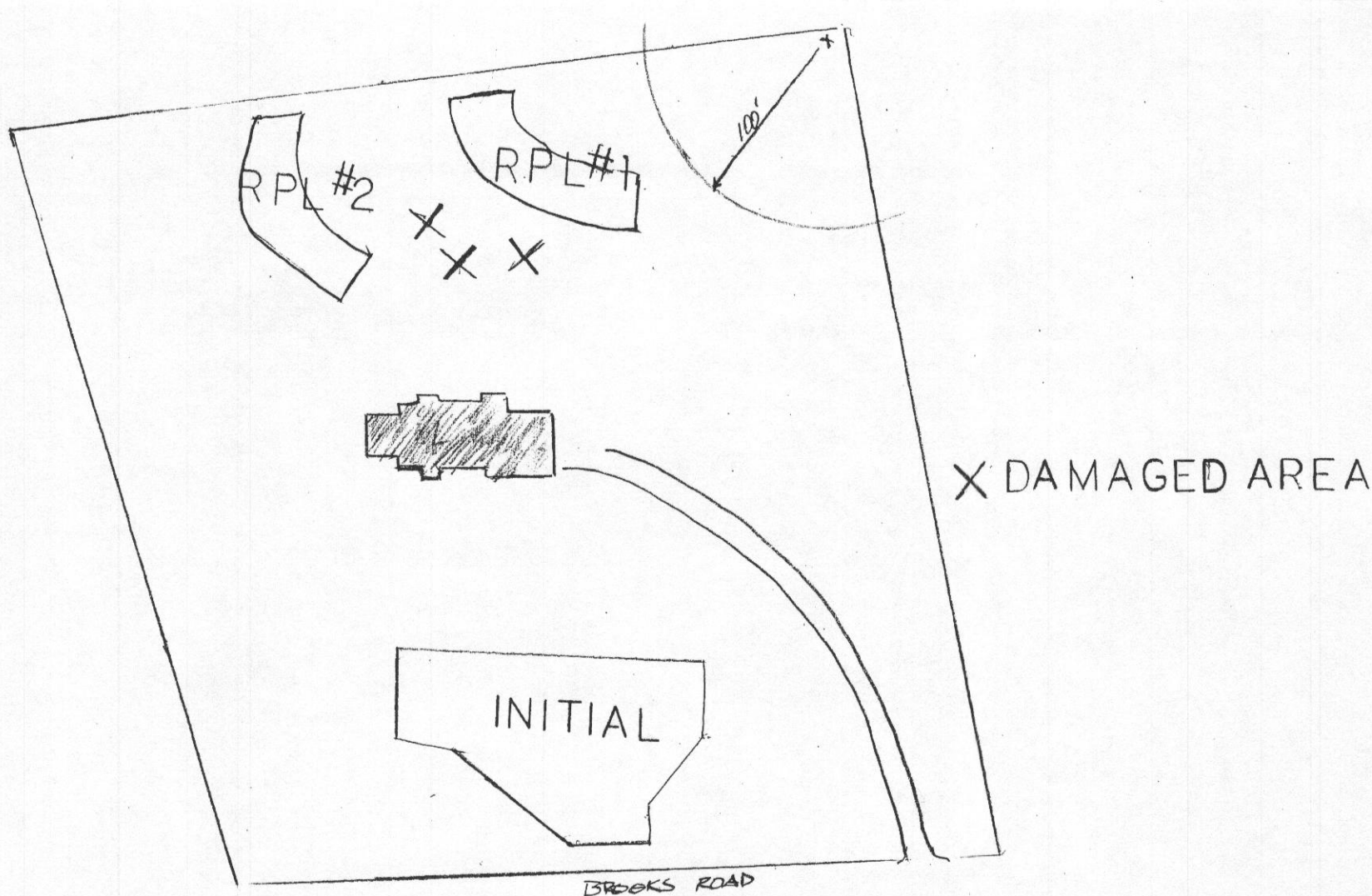
RECEIVED
HOWARD COUNTY HEALTH DEPARTMENT
ENVIRONMENTAL HEALTH DIVISION

WASTEWATER DISPOSAL SYSTEM
 OYEKAN PROPERTY, RAYMOND MORGAN SUBDIVISION LOT 4
 7145 BROOKS ROAD
 HIGHLAND, MARYLAND
 JUNE 2008

CONTENTS

PLATE	DESCRIPTION
1-10	PROJECT
2-10	SEPTIC DISPOSAL AREA / ADDITIONAL TESTING
3-10	DESIGN OF INITIAL AND REPLACEMENT FIELDS
4-10	DEEP TRENCH PROFILE/ SAND MOUND PLAN & PROFILE VIEW
5-10	2500 GAL. PUMP TANK/ PUMP SELECTION/ FLOAT INSTALLATION/ 'D' BOX
6-10	2000 GAL. SEPTIC TANK / SYSTEM ELEMENT ELEVATIONS
7-10	EFFLUENT FILTER
8-10	PUMP CURVE & CONTROL PANEL (INITIAL FIELD)
9a -10	INITIAL FIELD PLAN VIEW (DOWN CONTOUR FROM HOME)
9b -10	REPLACEMENT FIELD(S) PLAN VIEW (UP CONTOUR FROM HOME)
10 - 10	INITIAL FIELD DISPOSAL SYSTEM - PROFILE VIEW
	Vertical scale: 1" = 4'
	Horizontal scale: 1" = 30'





Design for: **Oyekan Property**
7145 Brooks Road, Howard County, Md.
Tax Map 40, Parcel #254, Raymond Morgan Subdivision Lot 4

REVISION - JUNE 2008

The site wastewater plan, approved June 2006 was compromised before it could be installed, by encroachment of incorrectly located landscaping work. As a result, the approved disposal area was declared invalid by HCBEH [Howard County Bureau of Environmental Health]. Additional disposal area was declared necessary to validate the site's 900 gallon/day wastewater Design Flow. Two Sand Mound replacement areas had been rendered unfit, and further testing on the property (4/30/2008) was conducted to supplant them. Additional disposal area (s) was found in the form of a deep trench field **now designated as the Initial Disposal Field**, and a replacement area Sand Mound **designated as RPL #2**. The Sand Mound designated previously, as Initial, on the approval of June 2006 is **now designated RPL #1**.

Testing April 30, 2008 located two field areas, see Plan View drawing. Prior consultation with Mr. Barry Glotfelty, MDE Regional Sanitarian, and HCBEH, advocated a repositioning of the [former Initial] RPL #1 basal area more upslope from the graded (30% +slope) terrace to ensure correct functioning of that sand mound, should the replacement ever be required. The reorientation is accomplished by this revision.

The three fields which constitute the SDA (Septic Disposal Area) provide an aggregate of 21,000ft² of disposal area [Initial: 11,400ft², RPL#1: 4,840ft², RPL#2 4,760ft²]

The 2000 gal. Septic Tank, and 2500 gal. Pump Tank were installed subsequent to the previous plan approval. The forcemain to the previous SDA(Initial field) had not been installed (2008), but has been re- routed to the 2008, Initial (deep trench) Disposal Field. The deep trench field is dosed by pump demand to a distribution box which gravity feeds the four trenches of the field. A six event /24 hrs. regime will require an 150 gallon effluent volume per dose.

When it is decided to shut down the Initial Field and dose to a replacement sand mound it will be necessary to change out the effluent pump originally installed to a pump with greater capacity

(James) Oyekan
 7145 Brooks Rd, Highland MD

Sheet Title:
 WASTEWATER
 SYSTEM
 PLAN

Sheet#
 WWT-1
 1 of 10 Sheets



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776
 (410) 875-9370 Office

(410) 635-2883 Fax
 H. Dale Gray, Principal

6/24/08

scale:

Percolation Certification Testing (Revised)
 April 30, 2008
 7145 Brooks Road
 Highland Maryland

Ref. Prior Certification Tri-County Surveys, INC. report, 9/9/05

Sand Mound Testing - April 2008 - property, upper left quadrant

SM #8 /08 --passed:(23.8 minutes/inch @ 15" depth) + 60% rock @ 6'
 SM #9 /08 --passed:(51.7 minutes/inch @ 19" depth) + 50% rock @ 4'
 SM #10 /08--passed:(20 minutes/inch @ 13" depth) hard bottom @ 5'
 @ < 60 minutes/inch, testing qualifies area for conventional sand mound sizing

Conventional Trench Testing - April 2008 - property, upper left quadrant

TT 10 -- Failed: 75% rock @ 3'

TT 11-- stopped test (rock)

TT 12 -- Failed: 75% rock @ 1'

TT 13 -- Failed: 50% rock @ 8"

Deep Trench Tests (minutes/inch rate) - lower left quadrant (house/ front)

Test (Point)	Depth/Time (pipe)	(trench floor)	Depth/Time (4' saturation zone)	Pass/ Fail
Test A	5.5' >30 min.	12'	-	F
	6.5' >30 min.	12'	-	F
	7.0' >30 min.	12'	-	F
Test B	5.5' :10 min/in.	9'	16' checked @ +2 min/in	P
			13' checked @ + 2 min.	P (all)
Test C	6.5' >30 min/in.	9'	-	F
	7.5' :20 min/in.	9'	-	P
	-	9'	13' checked @ +2 min/in	P
Test D	7.5' :20 min/in.	10'	14' checked @ +2 min/in	P(all)
Test E	7.7' :22 min/in.	11'	15' checked @ + 2 min.	P (all)

Per HCBEH, avg. Perc Rate:18 minutes/inch [0.6 gpd/ft² loading rate]

(James) Oyekan
 7145 Brooks Rd, Highland MD

Sheet Title:
 WASTEWATER
 SYSTEM
 PLAN

Sheet#

WWT-1
 2 of 10 Sheet



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS
 P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax
 H. Dale Gray, Principal

6.124.08

date:

INITIAL DISPOSAL FIELD

Deep Trench Configuration (New, Tested April 2008) [see Test results Page 2]
Perc. Rate : 18 minutes/inch [loading rate = 0.6 gal./day/ft²]
Stone Depth: 3.1ft, average depth below delivery pipe

Note: Test 'C' pipe depth was satisfactorily tested at 7.5' but provided a reduced stone depth figure of 1.5'. To avoid this reduced stone depth value, a minimum separation distance of 25' from 'C' to any trench structure was maintained.

Tile Field (TF) length required: 900 gpd/flow ÷ 0.6 gpd/loading rate = 1500 ft² trench bottom
or 1500 ft² ÷ 2' trench width = 750' linear length

DEEP TRENCH (DT) CONFIGURATION

$\frac{w+2}{w+1+2d} = \% \text{ equivalent of Tile Field length needed}$
 $\frac{2+2}{2+1+2 \times 3.1} = \frac{4}{9.2} = 43.4\% \text{ say } 43\%, \text{ thus } 750 \text{ lin. ft (TF)} \times 43\% = 323 \text{ lin ft DT required}$

REPLACEMENT DISPOSAL FIELDS (RPL) [Sand Mounds] #1 AND #2

RPL #1 (approved June 2006) Alternative Sand Media
Sand Mound Test (SM #1, #2, & #3) Results for RPL #1 were validated by the Perc Certification of Sept. 9, 2005. The Disposal Bed of this Mound lies along the 485' Elev. contour. The Mound Basal area has been relocated up the 8% sloped contour, several feet as the result of applying the field run survey data of JBA & Assoc. to the approved Plan of 6/2006. Basal area relocation of several feet does increase separation from the steeper, adjacent landscaped slopes (+25%) whose proximity had been of concern to MDE.

RPL #2 (New, Tested April 2008) Alternative Sand Media [see test results Page 2]
Sand Mound Tests (SM #8, #9, & #10) locate the Disposal Bed of this Mound along the 472' Elev. contour

7145 BROOKS ROAD
SM REPLACEMENT #2
SLOPE 11%

TABLE 3.1

EQUATIONS FOR CALCULATING SAND MOUND DIMENSIONS

Absorption bed ft.² (A × B) = $\frac{900 \text{ GPD}}{\text{Design flow}} = 900 \text{ ft.}^2$
1.0 gpd/ft.² (ALTERNATIVE SAND MEDIA)

Bed length (B) = 90 ft. (21 ft. to 101 ft. dependent on site)

Bed width (A) = $\frac{\text{Bed}}{B} = \frac{900 \text{ ft.}^2}{90 \text{ ft.}} = 10 \text{ ft.}$ (15 ft. or less)

Upslope sand fill depth (D) = 48 in. - Z in. = 24 in. (12 in. min.)

Downslope sand fill depth (E) = [12 A × % slope] + D in. = 312 in.

Cap + topsoil at bed center (H) = 18 in.

Cap + topsoil at bed edge (G) = 12 in.

Total Bed Depth (F) = 10 in.

Sideslope setback (K) = $\frac{[(D + E) + 28 \text{ in.}]}{2} \times 3 = 176 \text{ in.}$ (14' 8")

Upslope setback (J) = (22 in. + D) × 3 × upslope corr. factor = 108 in. (9')

Downslope setback (I) = (22 in. + E) × 3 × downslope corr. factor = 266 in. (22' 2")

Total Width of Mound (W) = 12A + J + I = 494 in. (41' 2")

Total Length of Mound (L) = 12B + K + K = 1432 in. (119' 4")

✓ ADDITIONAL DATA [if required]
Basal Area [Ft²] required: $\frac{\text{design flow (gpd)}}{\text{Soil perc. rate (loading rate)}} = \frac{900 \text{ ft}^2}{0.6 \text{ gpd/ft}^2} = 1500 \text{ ft}^2$
Basal Area provided:
Slope: [Bed width + downslope setback] X Bed length
Level: Mound length X width
 $(10' + 21.75') \times 90' = 2850 \text{ ft}^2$
ADEQUATE

7145 BROOKS RD
SM REPLACEMENT #1
SLOPE 8%

TABLE 3.1

EQUATIONS FOR CALCULATING SAND MOUND DIMENSIONS

Absorption bed ft.² (A × B) = $\frac{900 \text{ GPD}}{\text{Design flow}} = 900 \text{ ft.}^2$
1.0 gpd/ft.² (ALTERNATIVE SAND MEDIA)

Bed length (B) = 92.3 ft. (21 ft. to 101 ft. dependent on site)

Bed width (A) = $\frac{\text{Bed}}{B} = \frac{900 \text{ ft.}^2}{92.3 \text{ ft.}} = 9.75 \text{ ft.}$ (15 ft. or less)

Upslope sand fill depth (D) = 48 in. - Z in. = 24 in. (12 in. min.)

Downslope sand fill depth (E) = [12 A × % slope] + D in. = 33.3 in.

Cap + topsoil at bed center (H) = 18 in.

Cap + topsoil at bed edge (G) = 12 in.

Total Bed Depth (F) = 10 in.

Sideslope setback (K) = $\frac{[(D + E) + 28 \text{ in.}]}{2} \times 3 = 170.04 \text{ in.}$ (14' 2")

Upslope setback (J) = (22 in. + D) × 3 × upslope corr. factor = 133 in. (11')

Downslope setback (I) = (22 in. + E) × 3 × downslope corr. factor = 219 in. (18' 3")

Total Width of Mound (W) = 12A + J + I = 469 in. (39')

Total Length of Mound (L) = 12B + K + K = 1448 in. (120' 8")

✓ ADDITIONAL DATA [if required]
Basal Area [Ft²] required: $\frac{\text{design flow (gpd)}}{\text{Soil perc. rate (loading rate)}} = \frac{900 \text{ GAL}}{0.6 \text{ gpd/ft}^2} = 1500 \text{ ft}^2$
Basal Area provided:
Slope: [Bed width + downslope setback] X Bed length
Level: Mound length X width
 $[9.75' + 18.25'] \times 92.3' = 2584$
AREA PROVIDED IS ADEQUATE

(James) Oyekan
7145 Brooks Rd, Highland MD

Sheet Title:
WASTEWATER
SYSTEM
PLAN

Sheet#

WWT-1

3 of 10 Sheets



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS
P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax
H. Dale Gray, Principal

WASTEWATER SUPPLY TO DISPOSAL

Wastewater will be delivered to the Initial Disposal Field by pump to efficiently distribute to the deep trench field structure.

FLOW ASSUMPTION: Seven Hour Day (2hr. Morning, 1 hr. Afternoon, and 4 hr Evening)
 900 gpd/ 7 hr. Period = 420 minutes - hence, 900 gpd+420 minutes = 2.2 gpm
 2.2 gpm X the peaking factor 4 = 8.8 gpm, anticipated peak flow
 8.8 gpm X flow factor of 110% = **9.57 gpm projected maximum flow rate**

DOSE ASSUMPTION:
 6 Dose Demand Profile: 900 gal.+ 6 events = 150 gal./event

TOTAL DYNAMIC HEAD [TDH]:

Static Lift /Pump to Discharge Point ("D" Box Inlet): Off 445', Inlet 433.5' [11.5' elev]
 Friction Head 1 1/4" Sh 40, PVC @10 gpm = 1.55'/100' pipe ft.
 Velocity: = 2.21 FPS

Pipe -189' 1 1/4" Sh40 PVC :	1.89X1.55	= 2.9'
Fittings	10 cpls @1.2	= 12
	1 Gate valve (1 1/2")	= 0.8
	2 - 90°ells @4	= 8
	20.8 pipe feet	= 0.31
		3.27'elev

Operating Head.....2.0' elev
Total Dynamic Head [6.23']

PUMP SELECTION (0 TDH / 63 gpm)

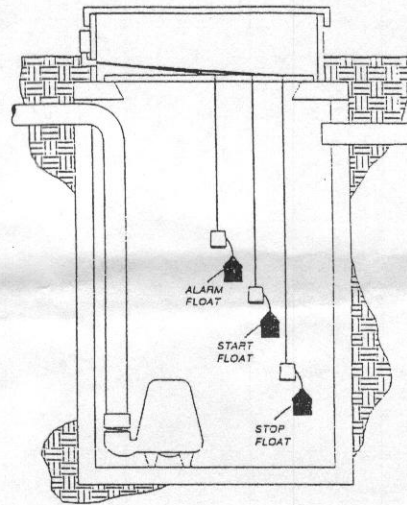
The selected Pump, Gould Model 3871 (EP05) has a light work load to serve the INITIAL Wastewater Disposal Field, however that pump could not serve RPL #1, or #2 Field locations. The major concern is to maintain sufficient pumping velocities in order to keep pipe walls scoured, and free of debris (FPS: 2.21 considered adequate velocity).

PUMP RUN TIME @ approx 63 gpm, an 150 gallon dose will require approximately two minutes and thirty seconds per event, or total of fourteen minutes per day.

rhombus
 TECHNOLOGY

INSTALLATION INSTRUCTIONS - Page 2
 Control switches with external weights

Figure 1 - Three Float Installation



MANUAL TESTING (To simulate simplex operation sequence).

For ease of installation and safety reasons, we recommend manual testing of float switch operation prior to attaching floats to discharge pipe in the pump chamber.

1. Make sure all float switches are in off position. SEE FIGURE 2.
2. Turn on power source. The control panel control switch should be on and the HOA switch should be in automatic position.
3. Tip stop float to on position.
4. While stop float remains tipped, tip start float to on position. At this point the pump and pump run light will turn on.
5. Return start float to off position. Return stop float to off position. Pump and pump run indicator light will now be off.
6. To test alarm operation, tip alarm float to on position. The red light and horn should be activated.

NOTE: UNIT SHOULD BE PERIODICALLY TESTED TO INSURE PROPER OPERATION.

Mounting Control Switches

CAUTION: Do not begin installation in pump chamber until all power source circuit breakers have been turned off. For added safety also turn off the control switch and the HOA switch. Failure to turn off power could result in serious or fatal electrical shock.

Cable Weight (Figure 2)

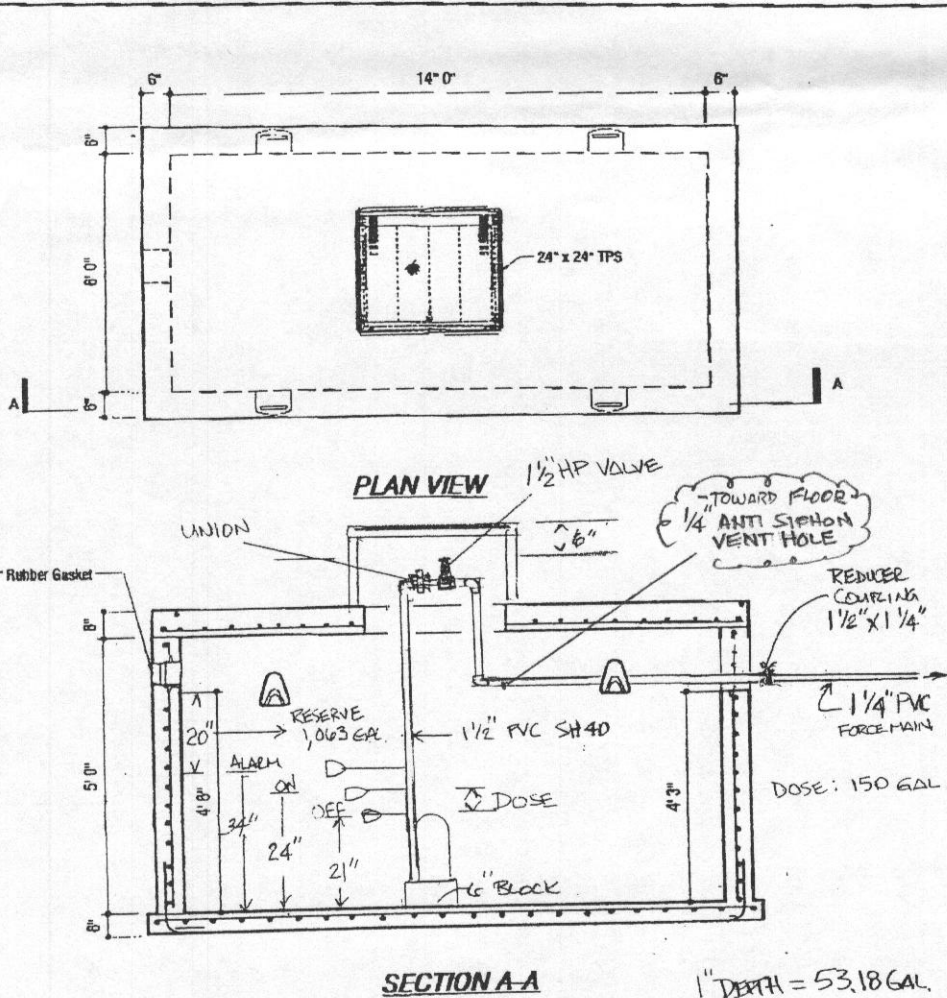
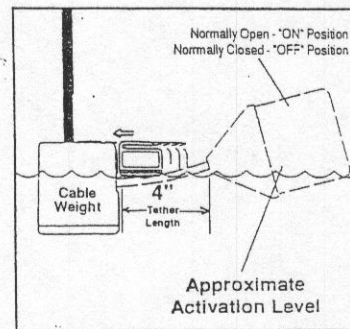
1. Lay cable in weight channel.
2. Align clip with weight groove and slide towards cable.
3. Snap clip snugly up to cable, manually moving clip to the tightest possible position.
4. Wire cable leads directly into control device.
5. Suspend unit at desired activation level. See Figure

Float Switch Specifications:

Sensor Float® control switch. HOUSING: 3.38 in. (8.58 cm.) diameter x 4.55 in. (11.56 cm.) long, high impact resistant, non-corrosive PVC plastic for use in liquids up to 140° F (60°C). CABLE: 16 gauge, 2 conductor S.O.W.-A (UL), SJOW (CSA) water resistant Neoprene.

ISSUE
 Printed in USA

Figure 2



DESIGN DATA & GENERAL NOTES

- (1) Concrete strength f'_c = 5,000 p.s.i. @ 28 days. Density = 150 pcf.
- (2) Cement - Portland Type III per ASTM C 150-92.
- (3) Admixtures & plasticizers per ASTM C 260-96 & C 494-92.
- (4) Reinforcing per ASTM A615, Grade 60, domestic. Min. 1-1/2" cover.
- (5) Top slab sealed on jobsite with butyl rope mastic.
- (6) Concrete is Air Entrained.

MB
 Mayer Bros., Inc.

6264 Race Road
 Elkridge, Maryland 21075
 Tel. 410.796.1434
 Fax. 410.796.1438
 www.mayerbrosprecast.com

**2500 Gallon Non-Traffic Tank
 KDAR**

INNOVA, LTD.

Dwg. No. M757

No Scale

June 6, 2006

(James) Oyekan
 7145 Brooks Rd, Highland MD

Sheet Title:
**WASTEWATER
 SYSTEM
 PLAN**

Sheet#

WWT-1
 5 of 10 Sheets

INNOVA, LTD

INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776

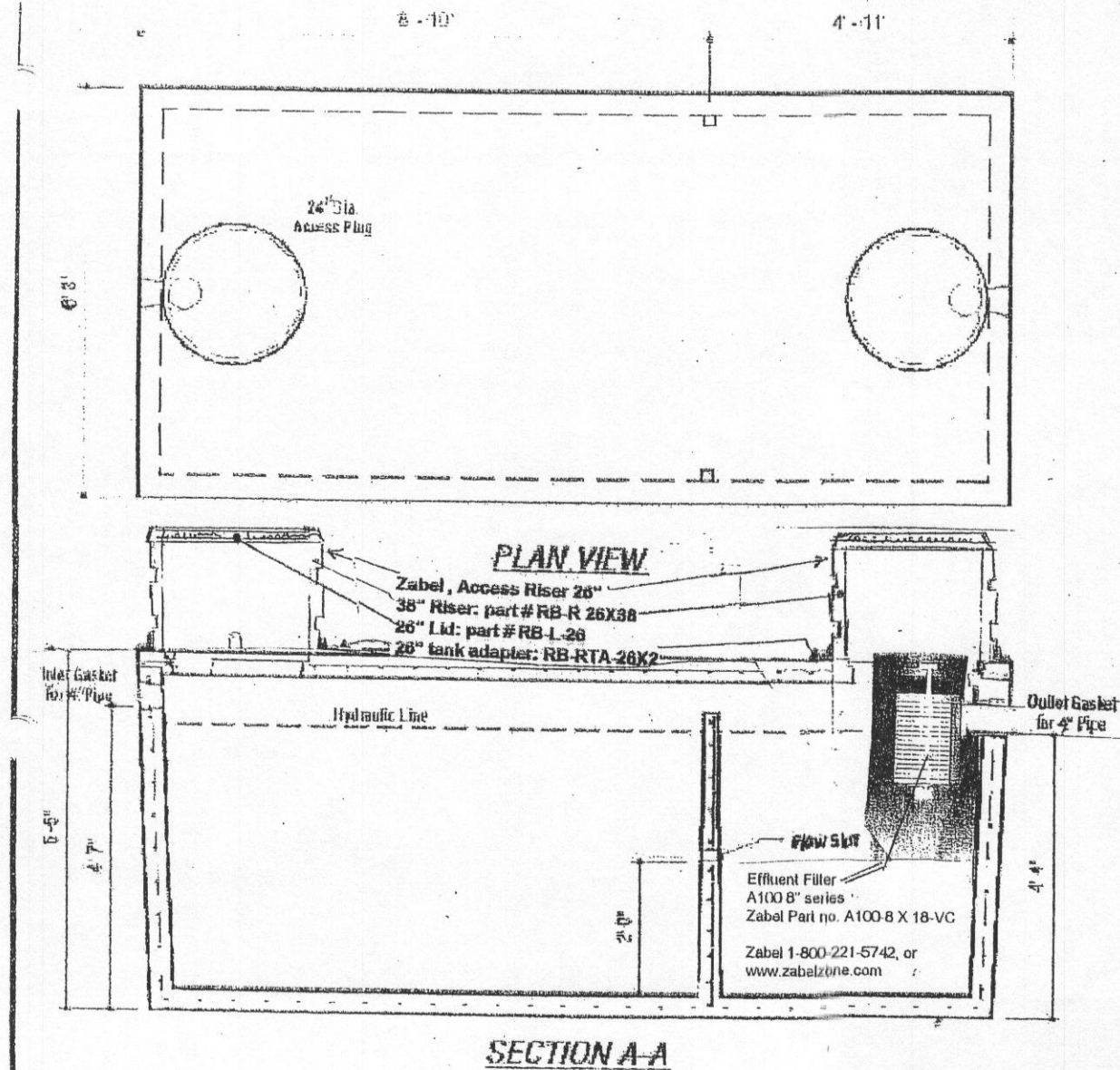
(410) 875-9370 Office

(410) 635-2883 Fax

H. Dale Gray, Principal

6.124108

Scale:



DESIGN DATA & GENERAL NOTES

- [1] Concrete strength: 4,000 p.s.i. 28 days. Density = 150 p.c.f.
- [2] Cement - Portland Type III per ASTM C 150-92.
- [3] Admixtures & plasticizers per ASTM C 260-86 & C 494-02.
- [4] Reinforcing per ASTM A 108 Min. 1/8" cover.
- [5] Top slab sealed with butyl rope mastic.
- [6] 4" wall, 4" base, & 5" top thickness.



8264 Race Road
Lanette, Maryland 21073
Tel. 410.796.5434
Fax. 410.796.1438

2,000 GALLON SEPTIC TANK 2-Compartment

Stock Item [Approx. 19,900 lbs.]

COMPONENT ELEVATIONS REVISION - JUNE 2008

Est. Surface Elevation	Element	Elevation	Est. Surface Elevation	Element	Elevation(s)
451.0'	House Exit	449.0'			
451.0'	2000 Gal Septic Tank (S/T).			TRENCHES (deep / 2'wide)	
	Top	449.4'	435.5'	T1	430.0' (pipe) 426.5' (floor)
	Inlet	448.6'	432.0'	T2	426.5' (pipe) 423.4 (floor)
	Outlet	448.35	429.5'	T3	422.0' (pipe) 419.5 (floor)
	Base	444.35'	425.0'	T4	417.5' (pipe) 414.0 (floor)
451.0'	2500 Gal Pump Tank (PT)			#1 REPLACEMENT MOUND	
	Top	449.5'		F/M turnup	482.5'
	Inlet	448.0	484.5'	Distribution Manifold	486.83'
	Base	443.5	484.5'	Mound Apex	488.96'
	Discharge	448.0'-450.0'	484.5'		
	Force Main [F/M] (3' burial)				
451.0'	Discharge	448.0'- 450.0'			
438.0'	Turn	435.5'			
435.0'	D BOX (inlet)	433.5'			
	(outlets) 433.33' (2" drop)				

Initial Mound - Bed Manifold/Laterals

481.5'	Turn up	480.0'
	Manifold	484.33'
481.5'	Mound Apex	486.0'

(James) Oyekan
7145 Brooks Rd, Highland MD

Sheet Title:
WASTEWATER
SYSTEM
PLAN

Sheet#

WWT-1

6 of 10 Sheets



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax
H. Dale Gray Principal

6/24/08

1" = 1'

date:



Zabel A100 Residential and Commercial Septic Tank Effluent Filter

Why do septic tanks need an A100 Zabel Filter?

Homes, schools, churches, shopping centers, apartment and rental properties all have two things in common: Extremely high wastewater peak rates and no way to predict what the users of those septic systems are likely to put down their toilets and drains. Every year thousands of drain fields fail and under go expensive repairs because they are clogged with solids that got out of an unfiltered tank.

What does an A100 Zabel Filter do?

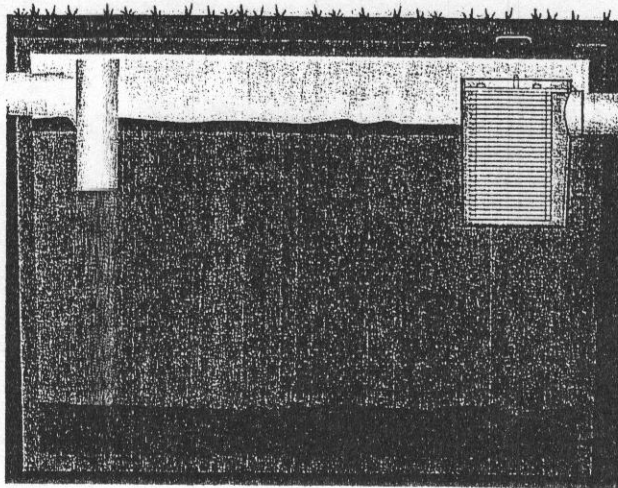
The A100 filter removes up to 90% of the solids and 45% of the BOD₅ from the waste stream. The Zabel A100 Residential and Commercial Effluent Filter removes all solids larger than 1/16" and protects the drain field from clogging with solids.

How often does the A100 Zabel Filter need Servicing?

The filter is virtually self cleaning. Anaerobic organisms on the filter discs partially digest lodged particles causing them to lose their buoyancy and fall to the bottom of the tank. In a standard residential installation the filter only needs to be cleaned when the tank is normally serviced. In commercial installations servicing will depend on the flow rate and solids loading characteristics of the wastewater.

What is the difference between a filter and a screen?

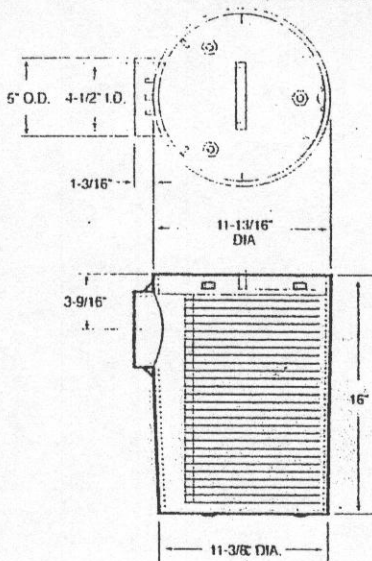
The larger filtration area of screens (usually five to eight times the size of filters) and the larger screen openings (1/8 inch for screens versus 1/16 inch for filters) are required because screens tend to plug easily and collapse. Zabel's exclusive patented disc dam design provides 198 lineal feet of filtration in a compact package 16 inches high and less than 12 inches in diameter making it the easiest filter on the market to install and service without sacrificing its ability to remove solids from the waste stream.



Call 1-800-221-5742 or Fax (502) 267-8801 for further information.

SPECIFICATIONS

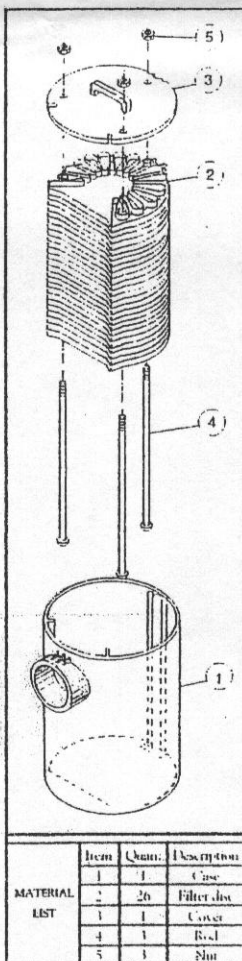
- APPLICATIONS:** The A100 is used in residential and commercial septic systems. It is effective in multi-family housing, rental property, schools, offices and everywhere wastewater has high suspended solids content.
- FLOW RATE:** 4,000 gpd per filter. Install two or more filters in a PVC or concrete manifold to achieve flows of 6,000 gpd or more. Check with Zabel for details.
- FILTRATION:** The 26 Disc Dams @ 1/16 inch provide 198 lineal feet of filtration.
- INSTALLATION:** The filter may be installed inside the tank or installed in a Zabel Container Assembly outside the septic tank.
- SERVICE:** Service residential installations whenever you pump the tank.



Material Specifications

Cases, Lids, Reducers Rigid Vinyl PVC 87171
Discs High Impact Polystyrene
Rod, Nuts High Density Polyethylene

U.S. Patent No. 4,710,295



Item	Quan.	Description
1	1	Case
2	26	Filter disc
3	1	Cover
4	1	Rod
5	1	Nut

Call 1-800-221-5742 or Fax (502) 267-8801 for further information.

Table 1
How the Filter Works

- The filter first stills the water exiting the Tank by forcing the effluent over a horizontal Weir. This prevents solids carried by wastewaters or gases from exiting the tank as in tanks with a conventional tee. Contained within the filter are over 61 lineal feet of weir dams.
- Second, the opening between each weir dam is only 1/16 of an inch. Solids any larger than 1/16 of an inch are trapped within the filter and tend to fall back to the bottom of the tank.
- Finally, microorganisms grow on the edges of the weir. The microorganisms not only reduce the size of the opening for solid particles exiting the tank, but they also tend to digest the solids passing over the weir, further treating the effluent.

James Oyekan
7145 Brooks Rd, Highland

Sheet Title:
WASTEWATER
SYSTEM
PLAN

Sheet#

WWT-
7 of 10 Sheets



INNOVA, LTD

INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax

U. Date Copy Received

6/28/08

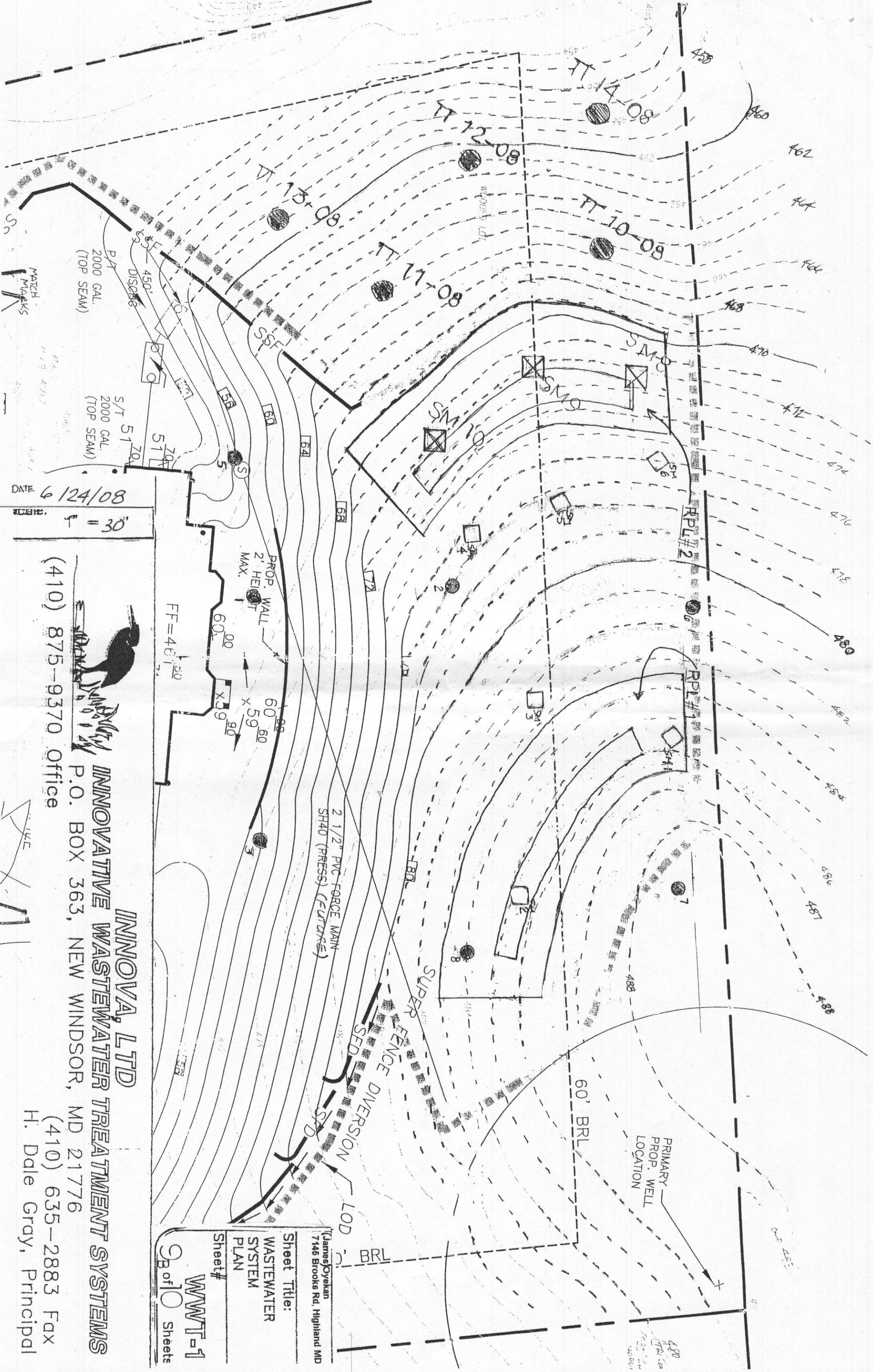
11e:



(410) 635-2883 Fax
H. Dale Gray, Principal

9 of 10 Sheets

DATE 6/24/08



DATE 6/24/08
SCALE 1" = 30'



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS

P.O. BOX 363, NEW WINDSOR, MD 21776

(410) 875-9370 Office

(410) 635-2883 Fax
H. Dale Gray, Principal

Sheet Title:
WASTEWATER
SYSTEM
PLAN

Sheet#
WWT-1
9 of 10 Sheets

(James) Dykeman
7145 Brooks Rd, Highland MD

DATE: 6/24/08



INNOVA, LTD
INNOVATIVE WASTEWATER TREATMENT SYSTEMS
P.O. BOX 363, NEW WINDSOR, MD 21776
(410) 875-9370 Office (410) 635-2883 Fax

(James) Oyekan
7145 Brooks Rd, Highland MD

Sheet Title:
WASTEWATER
SYSTEM
PLAN

Sheet#
WWT-1
10 of 10 Sheets

